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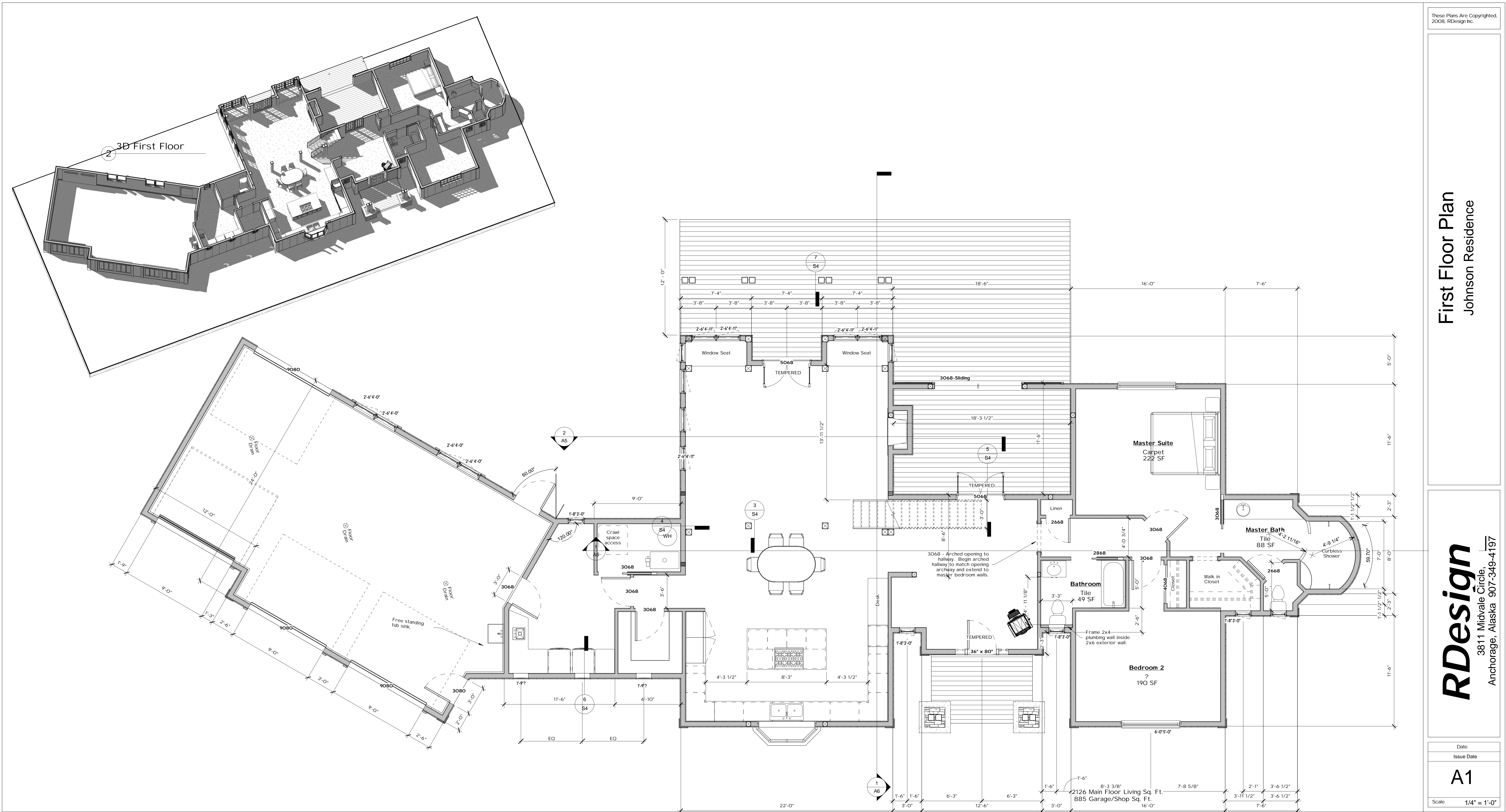
3D
Johnson Residence

RDesign
3811 Midvale Circle,
Anchorage, Alaska 907-349-4197

Date
Issue Date

A

Scale

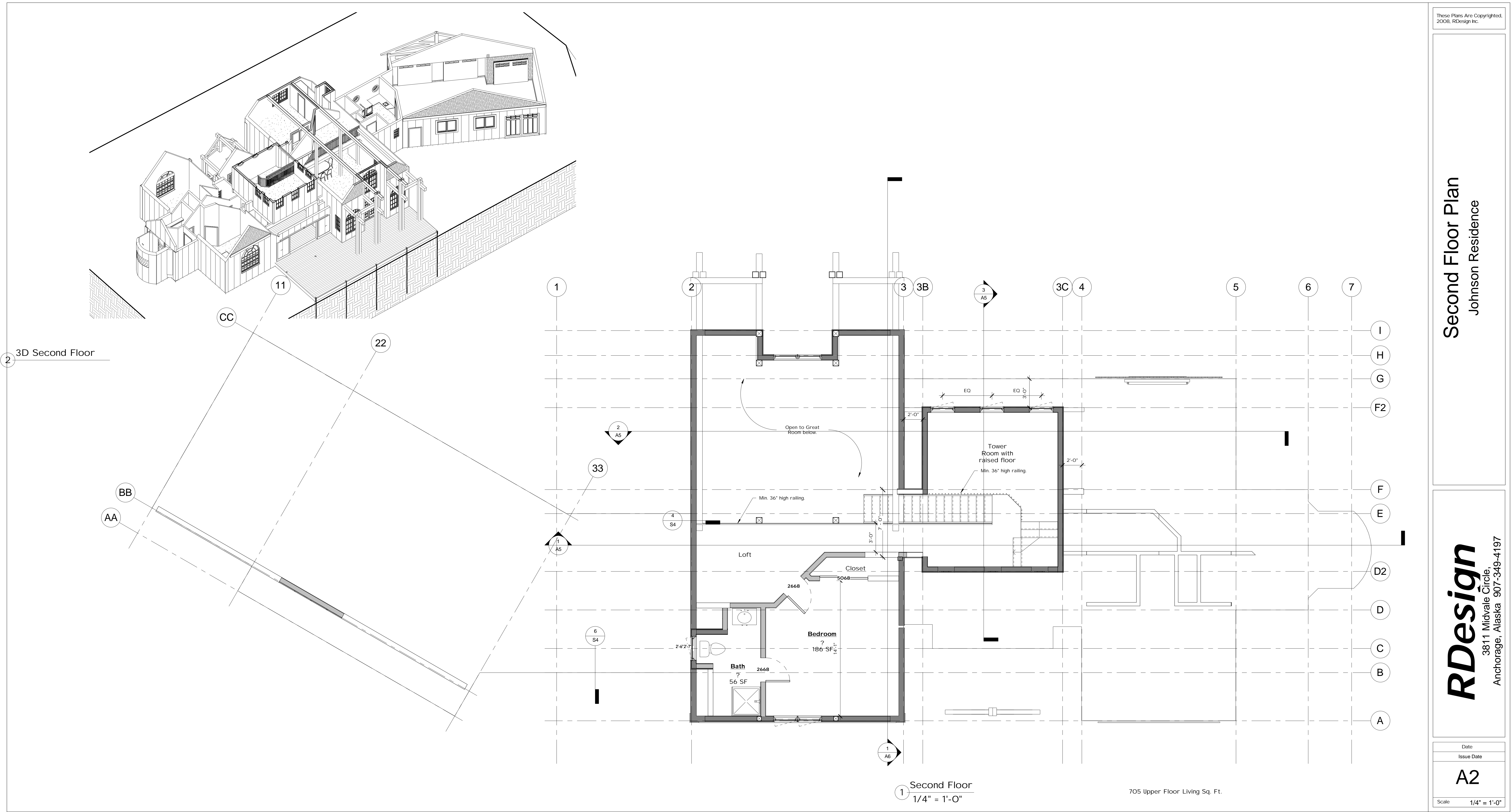


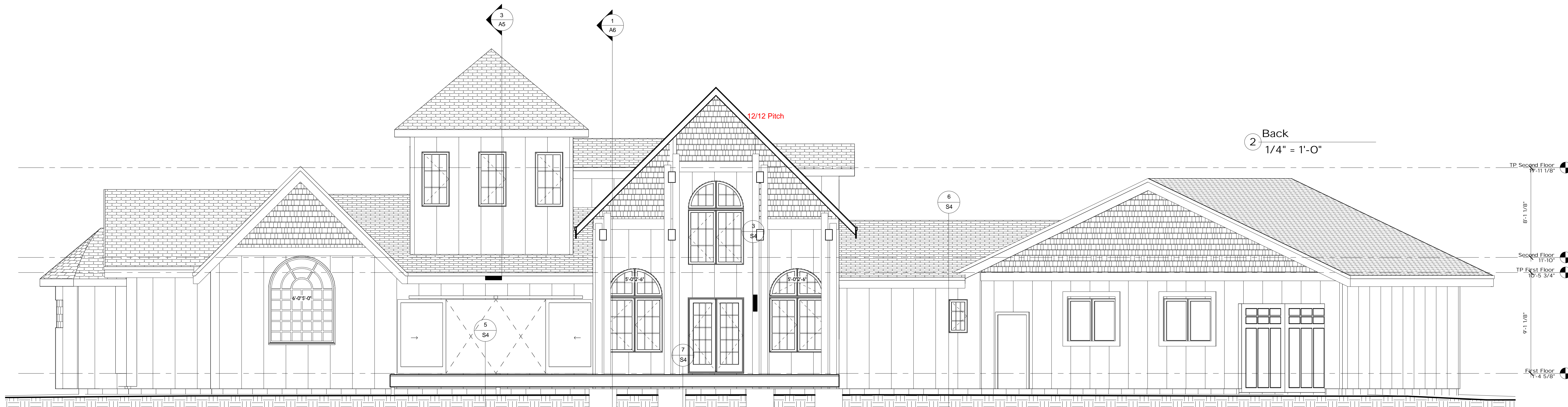
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First Floor Plan
Johnson Residence

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Date
Issue Date
A1
Scale 1/4" = 1'-0"





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Front and Back Elevations

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A3

Scale 1/4" = 1'-0"

Left and Right Elevations
Johnson Residence

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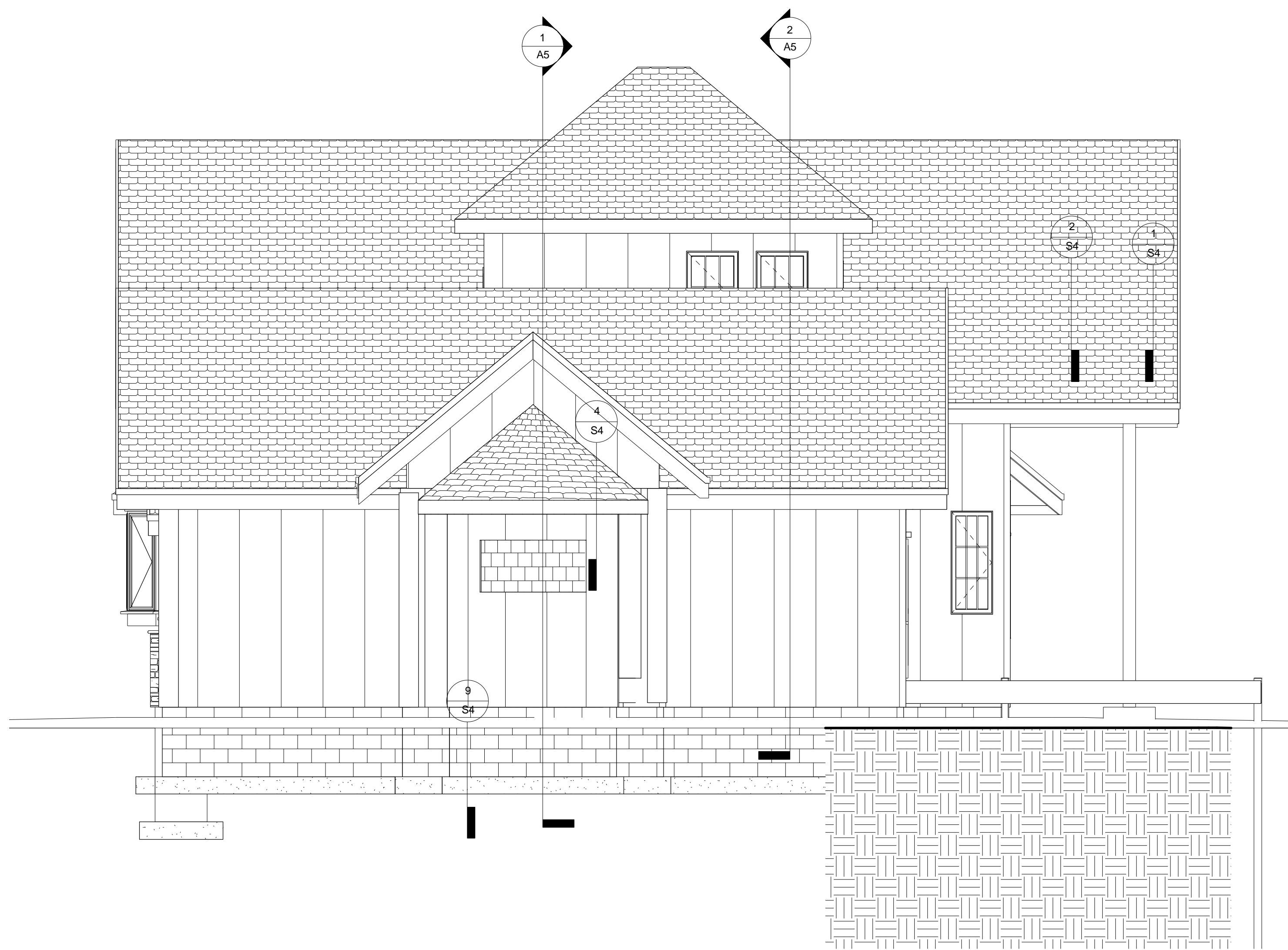
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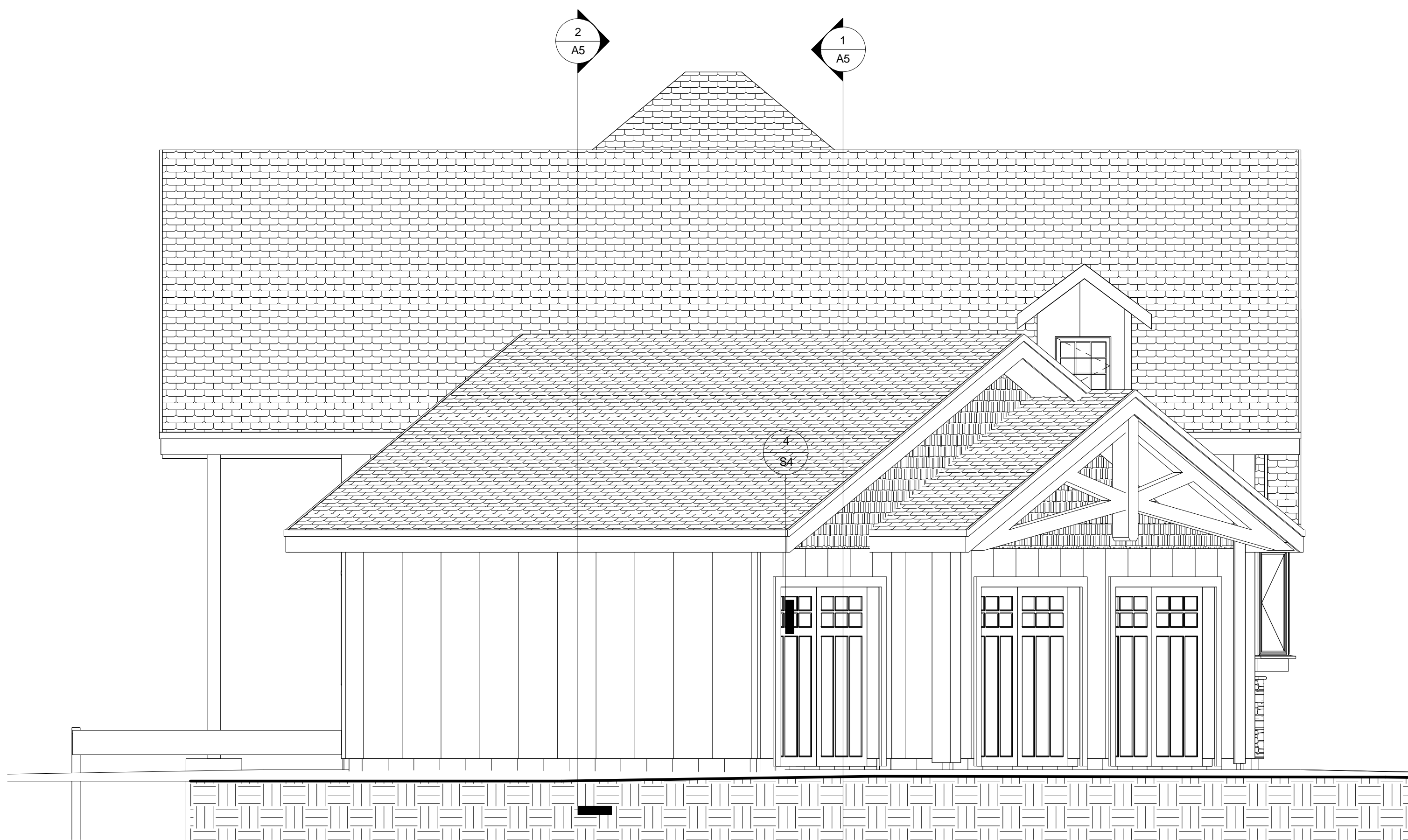
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Scale

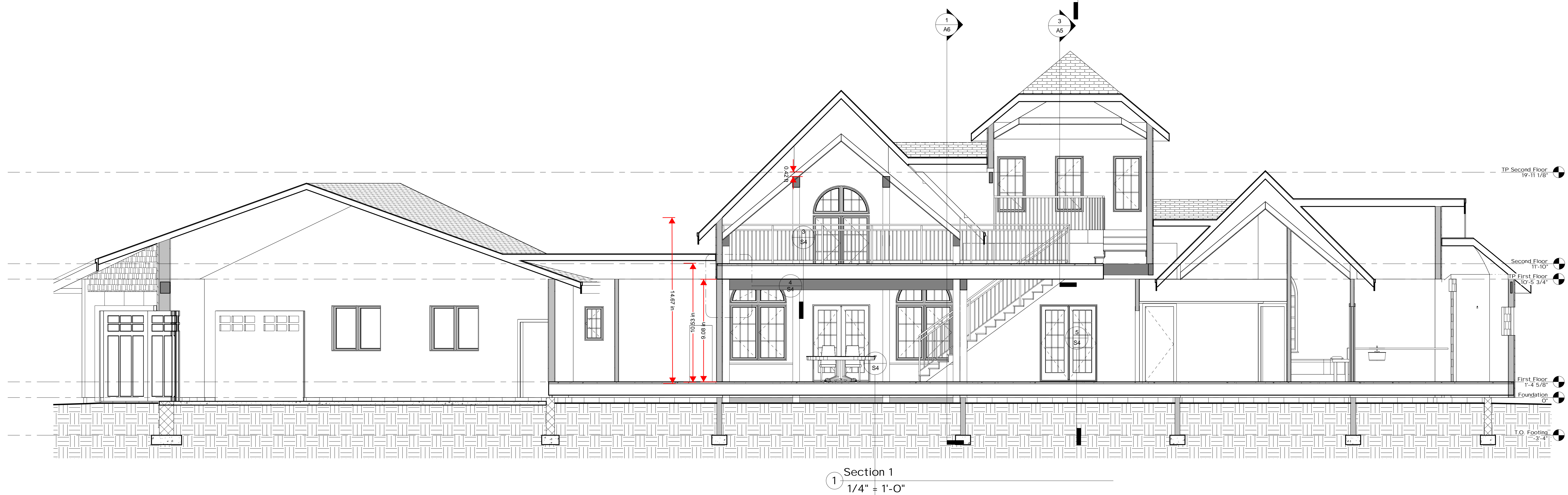
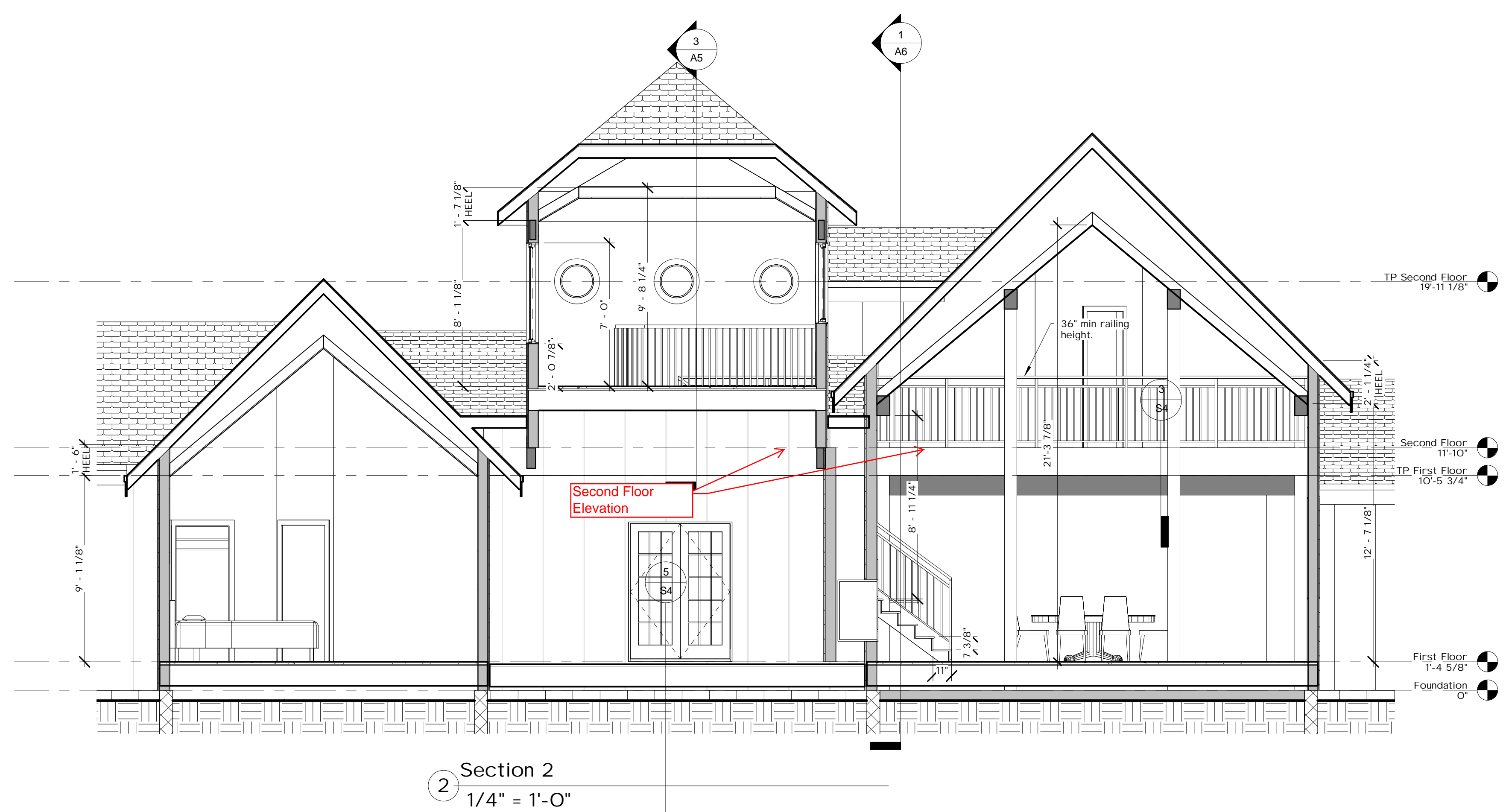
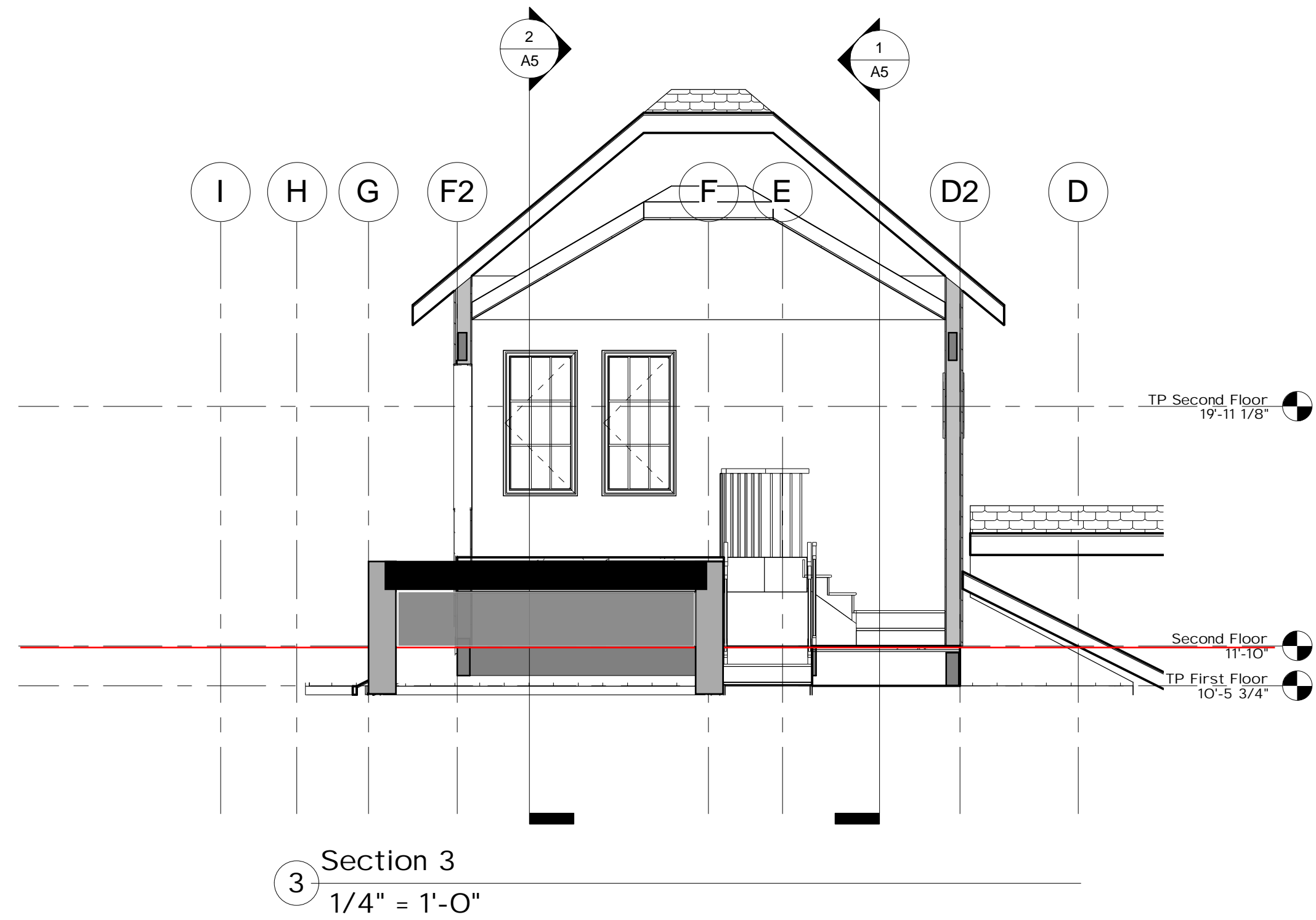
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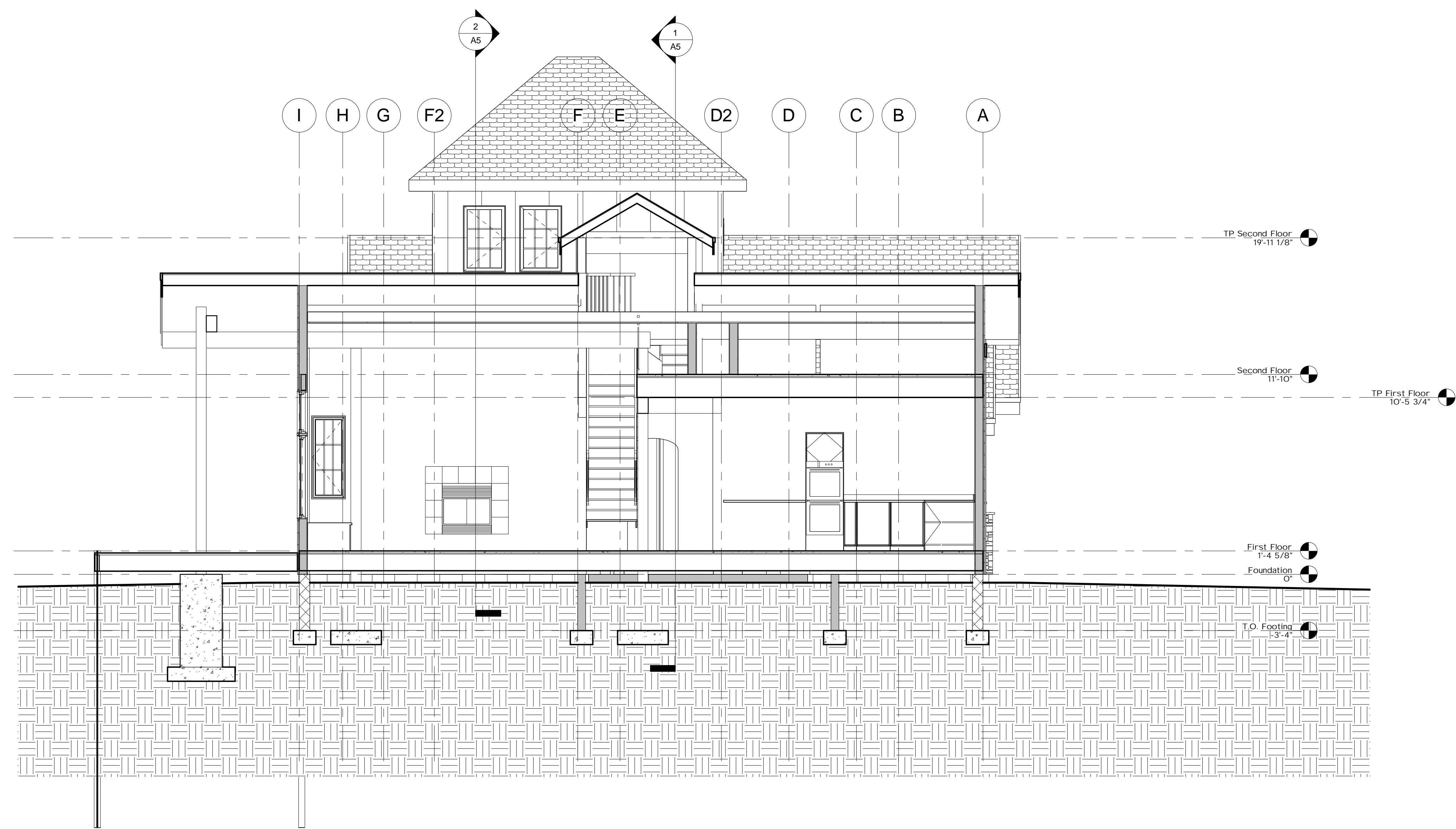


② Right
1/4" = 1'-0"



① Left
1/4" = 1'-0"





1 Section 7
1/4" = 1'-0"

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Building Sections
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Date
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Scale 1/4" = 1'-0"

(APPLY UNLESS NOTED OTHERWISE)

2009 EDITION OF T

LOADS:

1. BUI

7. ALL LOADS INDICATED ARE WORKING STRESS LOADS U.N.O.

1. THE S

10. ANY ENGINEERING DESIGN, PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW, SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF ALASKA.

1. FOUNDATION

- INTERIOR (CRAWL SPACE) FOOTINGS -----12" BELOW FINISHED GRADE

E POSITIVE DRAINAGE SLOPES, BOTH DURING AND

7. SEE ARCHITECTURAL/CIVIL DRAWINGS FOR EXTERIOR SLABS AND SIDEWALKS.

1. ALL COM

- DO NOT DROP CONCRETE MORE THAN FIVE FEET WITH OUT THE USE OF TREMIES.

CRETE SHALL BE FREE OF CHLORIDE. FOR I.C.F. WALLS, CONCRETE SLUMP AND

- SHALL NOT BE USED. RETEMPERING OF CONCRETE AFTER INITIAL SET IS NOT ALLOWED.
CURE EXPOSED CONCRETE PER ACI 301 FOR A MINIMUM OF 7 DAYS.

1. CONCRETE WALLS SHALL BE FORMED

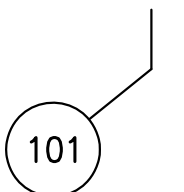


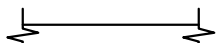





4. I.C.E. LEDGER CONNECTORS SHALL BE THE I.C.E./I. LEDGER CONNECTOR SYSTEM BY

1. REINFORCING STEEL

2. ACCURATELY PLACE OR SUPPORT ALL REINFORCING, WITH GALVANIZED METAL CHAIRS.

ALL OTHER PER LATEST EDITION OF ACI 318.

3. REINFORCING BAR SPACING GIVEN ARE MAXIMUM ON CENTERS. ALL BARS PER CRSI SPECIFICATIONS AND HANDBOOK. DOWEL ALL VERTICAL REINFORCE TO FOUNDATION WITH STANDARD 36-BE305EE. MAXIMUM 11-5/8" HIGHER. OTHERWISE, 12" MAXIMUM. 12" REQUIRED.

PLAN LEGEND		
SYMBOL	DESCRIPTION	REMARKS
	DETAIL CUTS SHOWN ON PLANS	FOUNDATION DETAILS ARE 100 SERIES NUMBERS. FRAMING DETAILS ARE 200 SERIES NUMBERS
	KEYNOTE	
	I.C.F. WALL U.N.O.	SEE PLANS AND SCHEDULES FOR SIZE AND REINFORCING
	WOOD STUD WALL U.N.O.	SEE PLANS AND SCHEDULES FOR SIZE
	HOLDOWN ANCHOR	SEE PLANS FOR LOCATION AND TYPE
	SLAB DEPRESSION/CHANGE IN ELEVATION	VERIFY DEPTH WITH ARCHITECTURAL DRAWINGS
	DIRECTION OF SLOPE	VERIFY SLOPE WITH ARCHITECTURAL AND/OR MECHANICAL DRAWINGS
	REVISION SYMBOL	
	OPENING	

ABBREVIATIONS			
A.B.C.	AGGREGATE BASE COURSE	LBS	POUNDS
A.F.F.	ABOVE FINISHED FLOOR	L.L.H.	LONG LEG HORIZONTAL
ALT	ALTERNATE	L.L.V.	LONG LEG VERTICAL
A.B.	ANCHOR BOLT	MFR('S)	MANUFACTURER('S)
B.F.F.	BELOW FINISHED FLOOR	M.C.J.	MASONRY CONTROL JOINT
B.O.B.	BOTTOM OF BEAM	MECH.	MECHANICAL
B.O.D.	BOTTOM OF DECK	N/A	NOT APPLICABLE
B.O.F.	BOTTOM OF FOOTING	N.F.S.	NON-FROST SUSCEPTIBLE
B.O.S.	BOTTOM OF STEEL	N.T.S.	NOT TO SCALE
BOT.	BOTTOM	O.C.	ON CENTER
BRG	BEARING	OPP	OPPOSITE (MIRRORED)
C.I.P.	CAST IN PLACE	P.A.F.	POWDER ACTUATED FASTENER
C.L.	CENTERLINE	P.C.	PRECAST CONCRETE
CLR	CLEAR	PCF	POUNDS PER CUBIC FOOT
CONC.	CONCRETE	PLF	POUNDS PER LINEAR FOOT
CONC. C.J.	CONCRETE CONTROL JOINT	PREFAB.	PREFABRICATED
C.M.U.	CONCRETE MASONRY UNIT	PSF	POUNDS PER SQUARE FOOT
CONN.	CONNECTION	PSI	POUNDS PER SQUARE INCH
CONT.	CONTINUOUS	REINF.	REINFORCING
D.L.	DEAD LOAD	SCH	SCHEDULE
DIA.	DIAMETER	SIM	SIMILAR
DN	DOWN	S.I.P.	STRUCTURAL INSULATED PANEL
DWG(S)	DRAWING(S)	SP	SPACES
(E)	EXISTING	STD	STANDARD
E.F.	EACH FACE	T & B	TOP AND BOTTOM
E.O.S.	EDGE OF SLAB	T.L.	TOTAL LOAD
EQ	EQUAL	T.O.B.	TOP OF BEAM
EQUIP.	EQUIPMENT	T.O.C.	TOP OF CONCRETE
EXP. BOLT	EXPANSION BOLT	T.O.D.	TOP OF DECK
E.J.	EXPANSION JOINT	T.O.F.	TOP OF FOOTING
E.W.	EACH WAY	T.O.L.	TOP OF LEDGER
F.F.E.	FINISHED FLOOR ELEVATION	T.O.M.	TOP OF MASONRY
GA	GAGE	T.O.P.	TOP OF PLATE
GALV.	GALVANIZED	T.O.S.	TOP OF STEEL
G.S.N.	GENERAL STRUCTURAL NOTES	T.O.W.	TOP OF WALL
GLB (GLULAM)	GLUED-LAMINATED BEAM	TYP	TYPICAL
HORIZ.	HORIZONTAL	U.N.O.	UNLESS NOTED OTHERWISE
I.B.C.	INTERNATIONAL BUILDING CODE	VERT.	VERTICAL
I.C.C.	INTERNATIONAL CODE COUNCIL	W.S.P.	WOOD STRUCTURAL PANEL
I.C.F.	INSULATED CONCRETE FORM	W.W.F.	WELDED WIRE FABRIC
K(KIP)	1000 POUNDS	W/	WITH
L.L.	LIVE LOAD	W/O	WITHOUT

6/24/15



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15-3A RESOLUTION POINTE
ANCHORAGE, ALASKA

**GENERAL STRUCTURAL NOTES AND
INTERPRETATION OF DRAWINGS**



revisions		
number	date	comment

[illegible]

engineer	W. OVIATT
draftsman	R. BRANCH
date	06/17/15

S1.0

sheet

GENERAL STRUCTURAL NOTES (CONTINUED)

(APPLY UNLESS NOTED OTHERWISE)

POST-INSTALLED ANCHORS:

- 1. INSTALL POST-INSTALLED ANCHORS ONLY AS INDICATED ON THE DRAWINGS AND/OR WITH SPECIFIC WRITTEN APPROVAL OF THE ENGINEER PRIOR TO INSTALLATION.
- 2. EPOXY ANCHORS IN CONCRETE SHALL BE THREADED ROD OR REINFORCING STEEL, INSTALLED WITH HIT-HY 200-R ADHESIVE BY HILTI PER I.C.C. REPORT ESR-3187.
- 3. EXPANSION ANCHORS IN CONCRETE SHALL BE KWIK BOLT TZ BY HILTI, INSTALLED PER I.C.C. REPORT ESR-1917.
- 4. SCREW ANCHORS IN CONCRETE SHALL BE TITEN HD BY SIMPSON, INSTALLED PER ICC REPORT ESR-2713.
- 5. THE CONTRACTOR MAY NOT USE SUBSTITUTES FOR POST-INSTALLED ANCHORS WITHOUT PRIOR APPROVAL OF THE ENGINEER.
- 6. SEE DRAWINGS FOR ANCHOR EMBEDMENT DEPTHS. INSTALL ANCHORS AS OUTLINED IN THE MANUFACTURER'S SPECIFICATIONS, UTILIZING PROPER SIZE AND TYPE OF DRILL AND PROPER HOLE CLEANING, DRIVING, AND TIGHTENING TECHNIQUES.

ROUGH CARPENTRY AND PLYWOOD:

- 1. ALL FRAMING PER IBC CHAPTER 23. FRAMING LUMBER SHALL COMPLY WITH THE 2005 EDITION OF THE NATIONAL DESIGN SPECIFICATION. MAXIMUM MOISTURE CONTENT SHALL NOT EXCEED 19 PERCENT. ALL SAWN LUMBER SHALL BE STAMPED WITH THE GRADE MARK OF AN APPROVED LUMBER GRADING AGENCY. ALL SAWN LUMBER SHALL BE OF THE FOLLOWING MINIMUM GRADES:

MEMBER TYPE	WOOD TYPE
JOISTS	
2x4 -----	D.F. STUD
2x6 OR LARGER -----	D.F. #2
BEAMS, LEDGERS, LINTELS AND TOP PLATES	
WIDTH 4" OR LESS -----	D.F. #2
WIDTH GREATER THAN 4" -----	D.F. #1
STUDS, PLATES AND BLOCKING	
2x4 -----	D.F. STND
2x6 OR LARGER -----	D.F. #2
POSTS	
4x4 -----	D.F. #2
4x6 OR LARGER -----	D.F. #2
6x6 OR LARGER -----	D.F. #1

- 2. ALL PLYWOOD SHALL CONFORM TO PS-1 OR APA PRP-108, SHALL HAVE AN EXTERIOR OR EXPOSURE 1 CLASSIFICATION AND SHALL BEAR THE STAMP OF AN APPROVED TESTING AGENCY. LAY UP PLYWOOD WITH FACE GRAIN PERPENDICULAR TO SUPPORTS AND STAGGER JOINTS. ON ROOFS WHERE PLYWOOD IS LAID UP WITH FACE GRAIN PARALLEL TO SUPPORTS, USE A MINIMUM OF 5-PLY PLYWOOD. ALL PLYWOOD SHALL BE OF THE FOLLOWING NOMINAL THICKNESS, SPAN/INDEX RATIO AND SHALL BE ATTACHED AS FOLLOWS UNLESS NOTED OTHERWISE:

USE	THICKNESS	SPAN/INDEX RATIO	EDGE ATTACHMENT	INTERMEDIATE ATTACHMENT
ROOF -----	5/8"	32/16 -----	10d AT 6" O.C. -----	10d AT 12" O.C.
FLOOR* - 3/4" T & G -----		48/24 -----	10d AT 6" O.C. -----	10d AT 10" O.C.

- * AT FLOORS, USE RING SHANK NAILS. IN ADDITION TO NAILING, GLUE ALL FLOOR SHEATHING TO FRAMING WITH AN APA AFG-01 QUALIFIED GLUE.
- 3. AMERICAN PLYWOOD ASSOCIATION PERFORMANCE RATED SHEATHING (OSB) MAY BE USED AS AN ALTERNATE TO PLYWOOD WITH PRIOR WRITTEN APPROVAL OF ARCHITECT. WHERE ROOF IS TO BE GUARANTEED, IT MAY NOT BE USED WITHOUT PRIOR APPROVAL FROM BUILT-UP ROOF SYSTEM MANUFACTURER. RATED SHEATHING SHALL COMPLY WITH I.C.C. REPORT NER-108, HAVE AN EXTERIOR OR EXPOSURE 1 CLASSIFICATION, AND SHALL HAVE A SPAN RATING EQUIVALENT TO OR BETTER THAN THE PLYWOOD IT REPLACES. ATTACHMENT AND THICKNESS (WITHIN 1/32") SHALL BE THE SAME AS THE PLYWOOD IT REPLACES. INSTALL PER MANUFACTURER'S RECOMMENDATIONS.
- 4. SEE TYPICAL DETAILS FOR ALLOWABLE NOTCHES AND HOLES IN STRUCTURAL FRAMING. DO NOT NOTCH, DRILL, OR SPLICE JOISTS, BEAMS, OR LOAD BEARING STUDS BEYOND THE DETAILS WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.
- 5. ALL LUMBER IN CONTACT WITH CONCRETE OR MASONRY FOUNDATIONS SHALL BE PRESSURE TREATED WOOD STAMPED BY AN APPROVED AGENCY. ALL UNTREATED LUMBER SHALL BE ISOLATED FROM CONCRETE OR MASONRY WITH 2 LAYERS OF 15# ROOFING FELT WITH OVERLAPPING JOINTS.
- 6. METAL FRAMING CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC. OR OTHER MANUFACTURER WITH CURRENT AND EQUIVALENT I.C.C. APPROVAL. ALL NAIL HOLES IN CONNECTORS SHALL BE FILLED WITH NAIL OF THE LARGEST SIZE INDICATED IN THE MANUFACTURER'S CATALOG U.N.O.

- 7. ALL NAILING NOT NOTED SHALL BE ACCORDING TO TABLE 2304.9.1 OF THE IBC. NAILS SHALL BE COMMON NAILS WITH THE FOLLOWING TYPICAL DIMENSIONS (PER ASTM F1667) UNLESS NOTED OTHERWISE:

(L = LENGTH, D = SHANK DIAMETER, H = HEAD DIAMETER)					
6d	L -----	2"	16d	L -----	3-1/2"
	D -----	0.113"		D -----	0.162"
	H -----	0.266"		H -----	0.344"
8d	L -----	2-1/2"	20d	L -----	4"
	D -----	0.131"		D -----	0.192"
	H -----	0.281"		H -----	0.406"
10d	L -----	3"	30d	L -----	4-1/2"
	D -----	0.148"		D -----	0.207"
	H -----	0.312"		H -----	0.432"

- 8. WHERE SCREWS ARE INDICATED FOR WOOD TO WOOD ATTACHMENTS, USE WOOD SCREWS WITH TYPICAL DIMENSIONS PER ANSI B18.6.1.

- 9. ALL FASTENERS IN CONTACT WITH TREATED LUMBER SHALL BE HOT-DIP GALVANIZED OR STAINLESS STEEL.

WOOD STAIRS:

- 1. WOOD STAIRS SHALL BE ASSEMBLED AND FABRICATED BY QUALIFIED CARPENTERS. ALL FRAMING IS PER IBC CHAPTER 23. MAXIMUM MOISTURE CONTENT SHALL NOT EXCEED 16%.
- 2. WOOD STAIR STRINGERS SHALL BE OF THE FOLLOWING MINIMUM SIZE AND SPECIES:
 - A. SOLID SAWN WOOD STRINGERS: 2x12 H.F. #2
 - B. ENGINEERED WOOD STRINGERS: 1-3/4" WIDE x 11-1/4" DEEP WITH A MINIMUM Fb = 2,600 PSI AND MINIMUM MODULUS OF ELASTICITY OF 1,700,000 PSI.
- 3. WOOD STAIR STRINGERS AS DETAILED ABOVE MAY BE USED FOR THE FOLLOWING SPANS AS MEASURED ALONG THE SLOPE OF THE STRINGER:
 - A. SOLID SAWN WOOD STRINGERS: (1) AT 12" O.C. - MAXIMUM SPAN = 8'-6". (2) AT 12" O.C. - MAXIMUM SPAN = 12'-6".
 - B. ENGINEERED WOOD STRINGERS: (1) AT 12" O.C. - MAXIMUM SPAN = 11'-6". (2) AT 12" O.C. - MAXIMUM SPAN = 14'-6".

IF THE PROJECT CONDITIONS EXCEED THOSE NOTED ABOVE, AN ENGINEERED DESIGN COMPLYING WITH THE IBC SHALL BE SEALED BY A REGISTERED ENGINEER AND SUBMITTED FOR REVIEW PRIOR TO ASSEMBLY.

- 4. MAXIMUM NOTCH (AS MEASURED PERPENDICULAR TO THE STRINGER PLANE) SHALL NOT EXCEED 5-3/4" FOR THE STRINGER SPANS SHOWN ABOVE.
- 5. STAIR TREADS SHALL COMPLY WITH THE FOLLOWING MINIMUM PROPERTIES: 3/4" PLYWOOD (OR OSB), 48/24 SPAN RATING OR 1x H.F. #2 FLAT WOOD DECKING.
- 6. ATTACHMENT TO ADJACENT WOOD OR MASONRY WALLS SHALL BE PER DETAILS. COORDINATE ATTACHMENT WITH ARCHITECTURAL FOR SOUND TRANSMISSION PRIOR TO CONNECTING STAIR STRINGERS TO ADJACENT WALLS.
- 7. FOR STAIR DIMENSIONS (INCLUDING RISE AND RUN) AND TREAD SURFACES, SEE ARCHITECTURAL DRAWINGS.

GLUED-LAMINATED BEAMS (GLU-LAM):

- 1. GLUED-LAMINATED BEAMS SHALL BE DOUGLAS FIR LARCH WITH THE FOLLOWING MINIMUM PROPERTIES: Fb = 2,400 PSI, Fv = 190 PSI, Fc (PERPENDICULAR) = 650 PSI, E = 1,800,000 PSI. BEAMS CANTILEVERING OVER SUPPORTS SHALL HAVE THE SPECIFIED MINIMUM PROPERTIES TOP AND BOTTOM.
- 2. CAMBER AS SHOWN ON DRAWINGS. IF NO CAMBER IS SPECIFIED, PROVIDE STANDARD CAMBER USING A RADIUS OF 3,500 FEET.
- 3. ALL BEAMS SHALL BE FABRICATED USING WATERPROOF GLUE. FABRICATION AND HANDLING SHALL BE PER LATEST AITC AND WCLA STANDARDS. BEAMS SHALL BEAR GRADE STAMP AND AITC STAMP AND CERTIFICATE. APPEARANCE GRADE BEAMS SHALL BE USED IF INDICATED ON ARCHITECTURAL DRAWINGS.

ENGINEERED LUMBER PRODUCTS:

- 1. LAMINATED VENEER LUMBER (LVL) PRODUCTS SHALL CONFORM TO APA/EWS PRL-501 AND I.C.C. ESR-1387. LVL MEMBERS SHALL BE "VERSA-LAM" 2.0E AS MANUFACTURED BY BOISE ENGINEERED WOOD PRODUCTS, OR APPROVED EQUIVALENT BY OTHER MANUFACTURER WITH CURRENT AND EQUIVALENT ICC APPROVAL.
- 2. LAMINATED STRAND LUMBER (LSL) PRODUCTS SHALL CONFORM TO ICC ESR-1387. LSL MEMBERS SHALL BE 1-1/4" "VERSA-STRAND" 0.8E OR "VERSA-STUD" 1.7E 2400 AS MANUFACTURED BY BOISE ENGINEERED WOOD PRODUCTS, OR APPROVED EQUIVALENT BY OTHER MANUFACTURER WITH CURRENT AND EQUIVALENT ICC APPROVAL.
- 3. I-SERIES FLOOR AND ROOF JOISTS SHALL BE BCI JOISTS MANUFACTURED BY BOISE, OR APPROVED EQUIVALENT BY ANOTHER MANUFACTURER, AND SHALL CARRY ICC APPROVAL FOR THE COMPOSITE SECTION. BRIDGING, BLOCKING, AND WEB STIFFENERS SHALL BE INSTALLED PER THE MANUFACTURER'S INSTRUCTIONS. NAILING NOT OTHERWISE SPECIFIED SHALL BE PER THE MANUFACTURER'S INSTRUCTIONS.

PREFABRICATED WOOD TRUSSES:

- 1. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE COMPLETE DESIGN, FABRICATION AND ERECTION PROCEDURES OF ALL TRUSSES, BRIDGING AND/OR BLOCKING PANELS, HANGERS, BRACING, ETC. FOR A COMPLETE INSTALLATION OF THE TRUSS SYSTEM. TRUSS CONFIGURATIONS ARE INDICATED ON THE DRAWINGS. ALL BRACING AND BRIDGING SIZES AND SPACINGS BY TRUSS MANUFACTURER IN ACCORDANCE WITH THE LATEST RECOMMENDATIONS OF THE TRUSS PLATE INSTITUTE.
- 2. TRUSSES SHALL BE DESIGNED AND FABRICATED IN ACCORDANCE WITH IBC CHAPTER 23 TO SUPPORT SELF WEIGHT PLUS LIVE LOAD, SUPERIMPOSED DEAD LOADS, AND LATERAL LOADS STATED IN THE GENERAL STRUCTURAL NOTES OR LOCATED ON PLANS. ROOF TRUSSES SHALL BE DESIGNED TO ACCOMMODATE A FUTURE TOP CHORD DEAD LOAD OF 300 POUNDS AT ANY LOCATION. THE UNIFORM LOADS DO NOT INCLUDE SPECIAL OR ADDITIONAL LOADS NOTED ON THE PLANS OR DETAILS. THE ROOF LOAD DURATION FACTOR IS 1.15.
- 3. LIMIT TOTAL LOAD DEFLECTIONS TO SPAN/240 AT SIMPLE SPANS U.N.O. LIMIT LIVE LOAD DEFLECTIONS TO SPAN/360 AT SIMPLE SPANS U.N.O. ALL TRUSSES SHALL BE CAMBERED FOR 1.5 TIMES THE DESIGN DEAD LOAD.
- 4. ADDITIONAL TRUSSES SHALL BE SUPPLIED AS REQUIRED TO SUPPORT MECHANICAL EQUIPMENT.
- 5. ALL CONNECTORS SHALL HAVE CURRENT I.C.C. APPROVAL. ALL TRUSS TO TRUSS CONNECTORS SHALL BE DESIGNED BY THE TRUSS MANUFACTURER. MULTIPLE TRUSS MEMBERS SHALL BE FASTENED TOGETHER TO ALLOW TRANSFER OF SHEAR AND TENSION FORCES (MINIMUM 200 PLF) AT PLYWOOD SHEATHING JOINTS AND TO PREVENT CROSS GRAIN BENDING OF TOP CHORDS. ATTACHMENT SHALL BE A CONTINUOUS 20 GAGE METAL PLATE OR OTHER APPROVED MEANS. METHOD OF ATTACHMENT SHALL BE INDICATED ON SHOP DRAWINGS FOR REVIEW.
- 6. WHERE PERMANENT BRACING OF TRUSS MEMBERS IS REQUIRED BY THE TRUSS DESIGN, IT SHALL BE ACCOMPLISHED BY THE FOLLOWING METHOD: THE TRUSSES SHALL BE DESIGNED SO THAT THE BUCKLING OF ANY INDIVIDUAL TRUSS MEMBER CAN BE RESISTED INTERNALLY BY THE STRUCTURE (E.G. BUCKLING MEMBER T-BRACING, L-BRACING, ETC.) OF THE INDIVIDUAL TRUSS. THE TRUSS INDIVIDUAL MEMBER BUCKLING REINFORCEMENT SHALL BE INSTALLED AS SHOWN ON THE TRUSS DESIGN DRAWING OR ON SUPPLEMENTAL TRUSS MEMBER BUCKLING REINFORCEMENT DIAGRAMS PROVIDED BY THE TRUSS DESIGNER.
- 7. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS, ERECTION DRAWINGS AND DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER FOR REVIEW PRIOR TO MANUFACTURE. CALCULATIONS AND SHOP DRAWINGS SHALL SHOW ANY SPECIAL DETAILS REQUIRED AT BEARING POINTS.
- 8. ALL FABRICATION SHALL BE PERFORMED ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION.

DEFERRED SUBMITTALS: (PER 2006 IBC 106.3.4.2)

- 1. FOR THE PURPOSES OF THIS SECTION, DEFERRED SUBMITTALS ARE DEFINED AS THOSE PORTIONS OF THE DESIGN THAT ARE NOT SUBMITTED AT THE TIME OF THE APPLICATION AND WHICH ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL WITHIN A SPECIFIED PERIOD.
- 2. DEFERRAL OF ANY SUBMITTAL ITEMS SHALL HAVE PRIOR APPROVAL OF THE BUILDING OFFICIAL. THE ARCHITECT OR ENGINEER OF RECORD SHALL LIST THE DEFERRED SUBMITTALS ON THE PLANS AND THE CONTRACTOR SHALL SUBMIT THE DEFERRED SUBMITTAL DOCUMENTS FOR REVIEW BY THE BUILDING OFFICIAL.
- 3. SUBMITTAL DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ARCHITECT OR ENGINEER OF RECORD A MINIMUM OF 30 DAYS PRIOR TO FABRICATION. THE DOCUMENTS SHALL BE REVIEWED FOR GENERAL CONFORMANCE WITH THE DRAWINGS. A COPY OF THE DEFERRED SUBMITTAL DOCUMENTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THEIR DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.
- 4. DEFERRED SUBMITTAL ITEMS:

REVIEWED FOR STRUCTURAL CODE COMPLIANCE

WOOD TRUSSES

6/24/15

JOHNSON RESIDENCE
15-3A RESOLUTION POINTE

ANCHORAGE, ALASKA

GENERAL STRUCTURAL NOTES

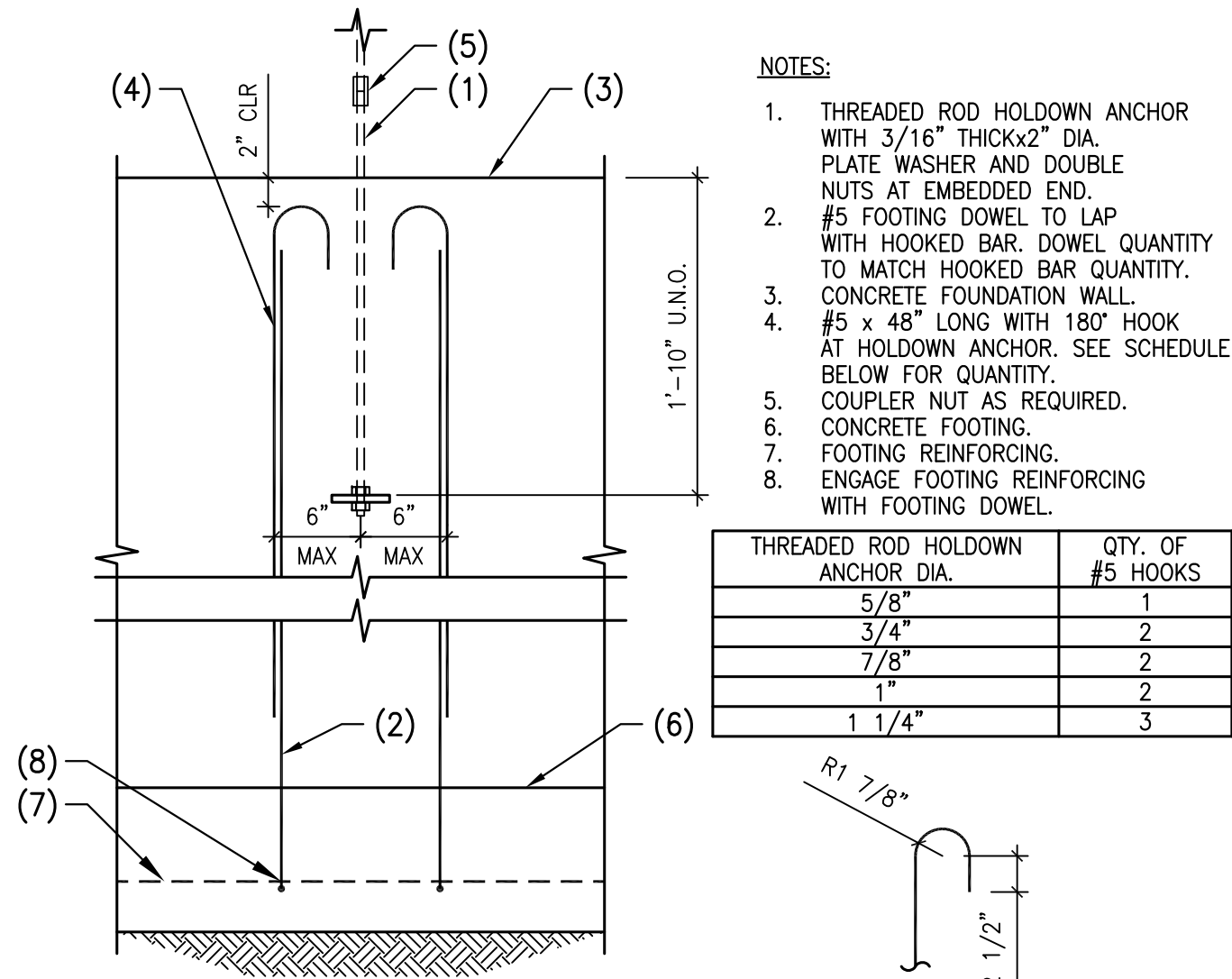


THE ENGINEER IS RESPONSIBLE ONLY FOR THE WORK ON THOSE SHEETS BEARING THE ENGINEER'S STAMP. THESE DOCUMENTS MAY NOT BE REPRODUCED IN ANY FORM WITHOUT THE EXPRESS WRITTEN CONSENT OF SCHNEIDER & ASSOCIATES STRUCTURAL ENGINEERS INC. SCHNEIDER & ASSOCIATES STRUCTURAL ENGINEERS INC. IS NOT RESPONSIBLE FOR ANY REVISIONS OR ADDITIONS TO THESE DRAWINGS UNLESS INITIALED OR AGREED TO IN WRITING BY THE ENGINEER.

revisions		
number	date	comment

project	215078
engineer	W. OVIATT
drafter	R. BRANCH
date	06/17/15

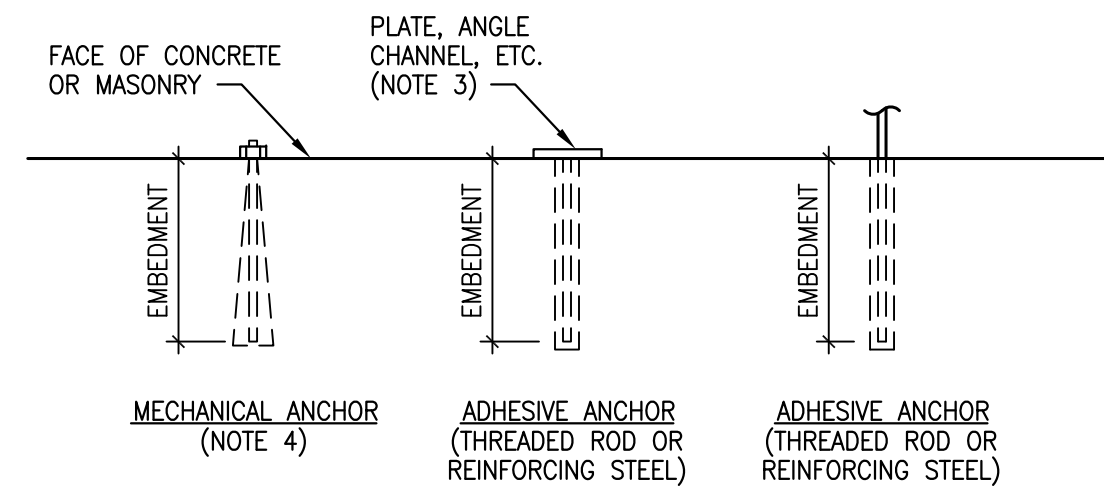
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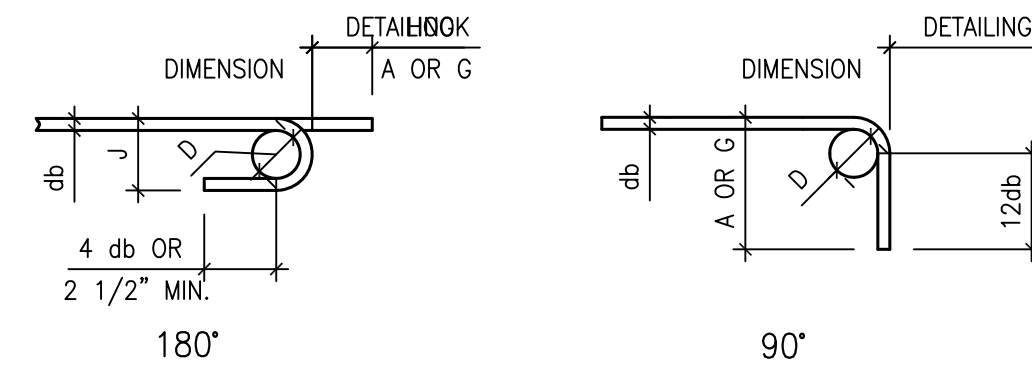
09 TYPICAL HOLDDOWN ANCHOR AT CONCRETE FOUNDATION WALL
SCALE: NOT TO SCALE 602-018-TYP

REINFORCING STEEL SIZE	REINFORCING STEEL EMBEDMENT LENGTH IN CONCRETE	REINFORCING STEEL EMBEDMENT LENGTH IN MASONRY
#3	3"	6"
#4	6"	8"
#5	6"	8"
#6	8"	8"
#7	8"	8"
#8	10"	8"
#9	12"	12"

ANCHOR DIAMETER	MECHANICAL ANCHOR EMBEDMENT LENGTH IN CONCRETE	MECHANICAL ANCHOR EMBEDMENT LENGTH IN MASONRY	THREADED ROD ANCHOR EMBEDMENT LENGTH IN CONCRETE	THREADED ROD ANCHOR EMBEDMENT LENGTH IN MASONRY
3/8"	3"	2 3/4"	4 1/2"	3 1/2"
1/2"	4"	3 1/2"	5"	4 1/2"
5/8"	5 1/4"	4 1/2"	6 3/4"	6"
3/4"	5 3/4"	5 1/2"	6 3/4"	7"
7/8"	---	---	7"	---
1"	8"	8"	8"	---
1 1/4"	---	---	10"	---

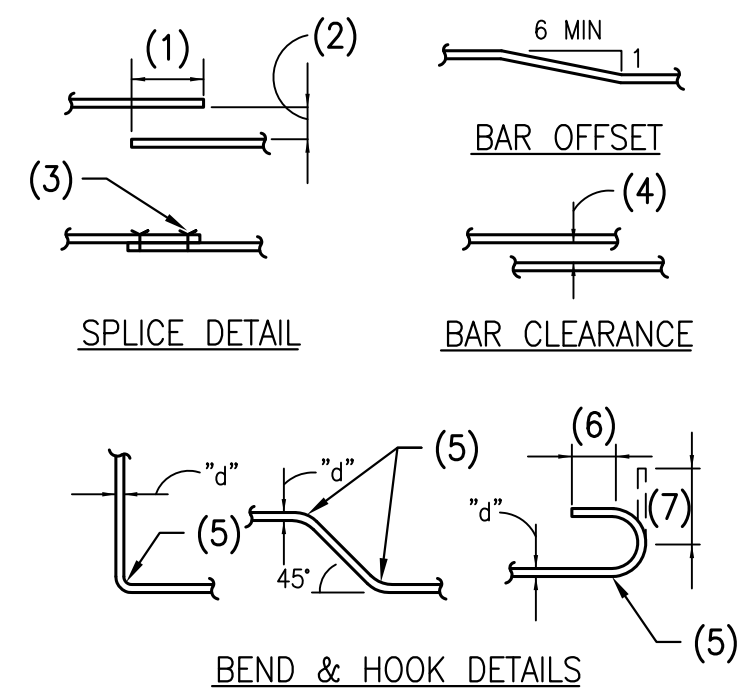


10 TYPICAL POST-INSTALLED ANCHOR AND REINFORCING STEEL SCHEDULE
SCALE: NOT TO SCALE 501-002-TYP



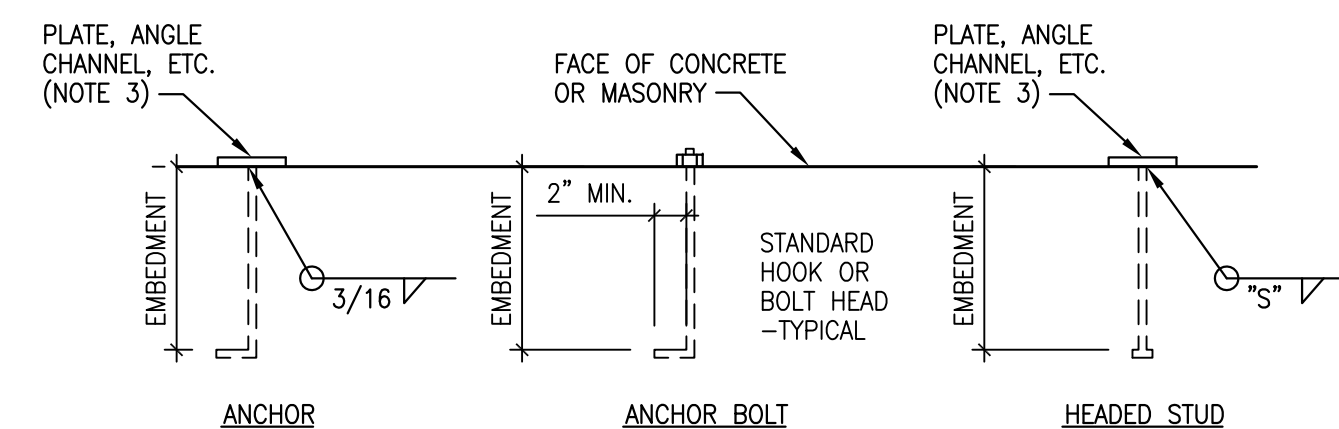
BAR SIZE	FINISHED BEND DIA. D, IN.	END HOOKS, ALL GRADES		
		180-DEG HOOKS		90 DEG HOOKS
		A OR G, IN.	J, IN.	A OR G, IN.
#3	2.25	5	3	6
#4	3	6	4	8
#5	3.75	7	5	10
#6	4.5	8	6	12
#7	5.25	10	7	14
#8	6	11	8	16
#9	9.5	15	11.75	19
#10	10.75	17	13.25	22
#11	12	19	14.75	24
#14	18.25	27	21.75	31
#18	24	36	28.5	41

06 TYPICAL REINFORCING HOOK SCHEDULE
SCALE: NOT TO SCALE 200-045-TYP

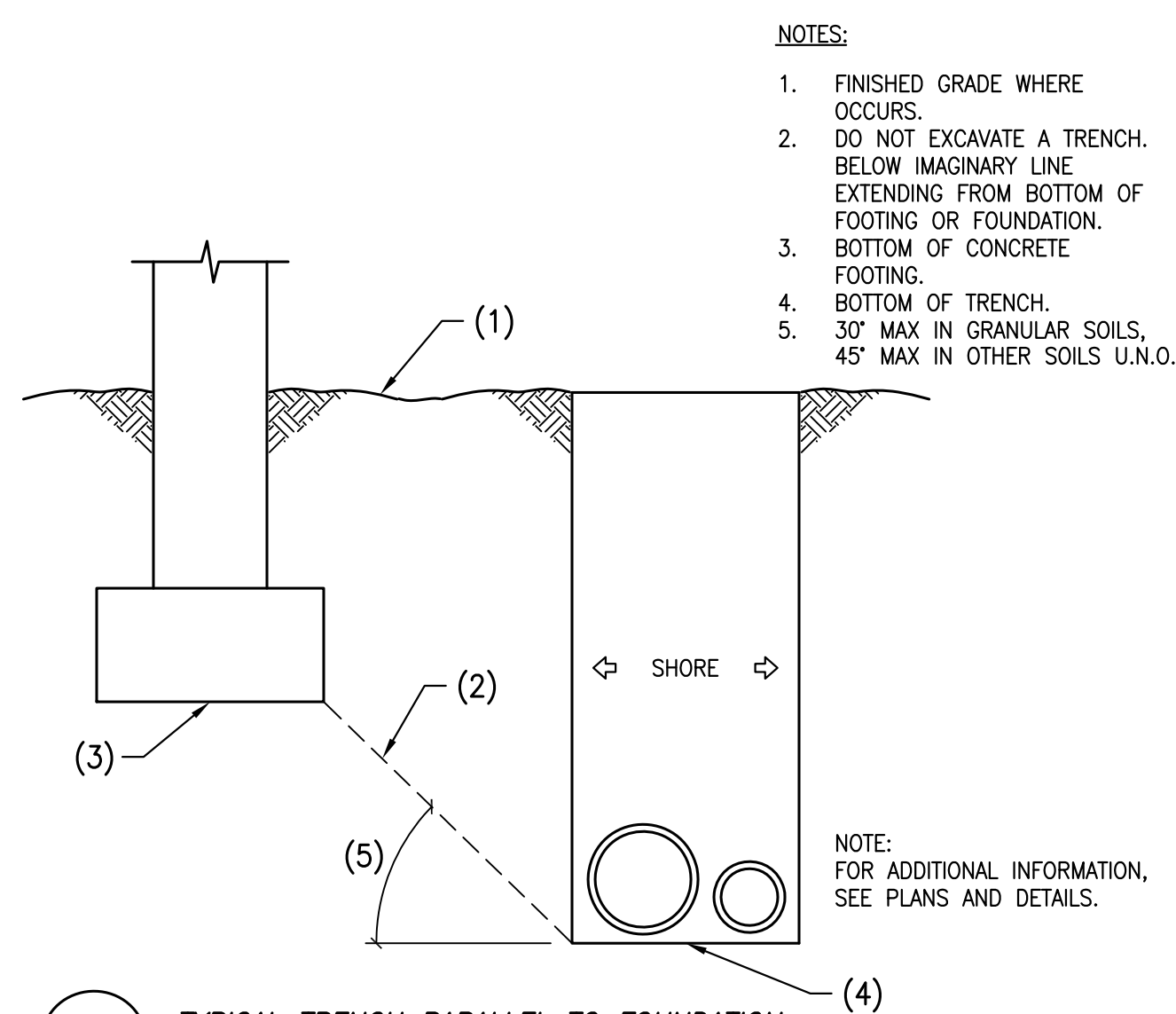


07 TYPICAL CONCRETE REINFORCING BAR DETAILS
SCALE: NOT TO SCALE 200-050-TYP

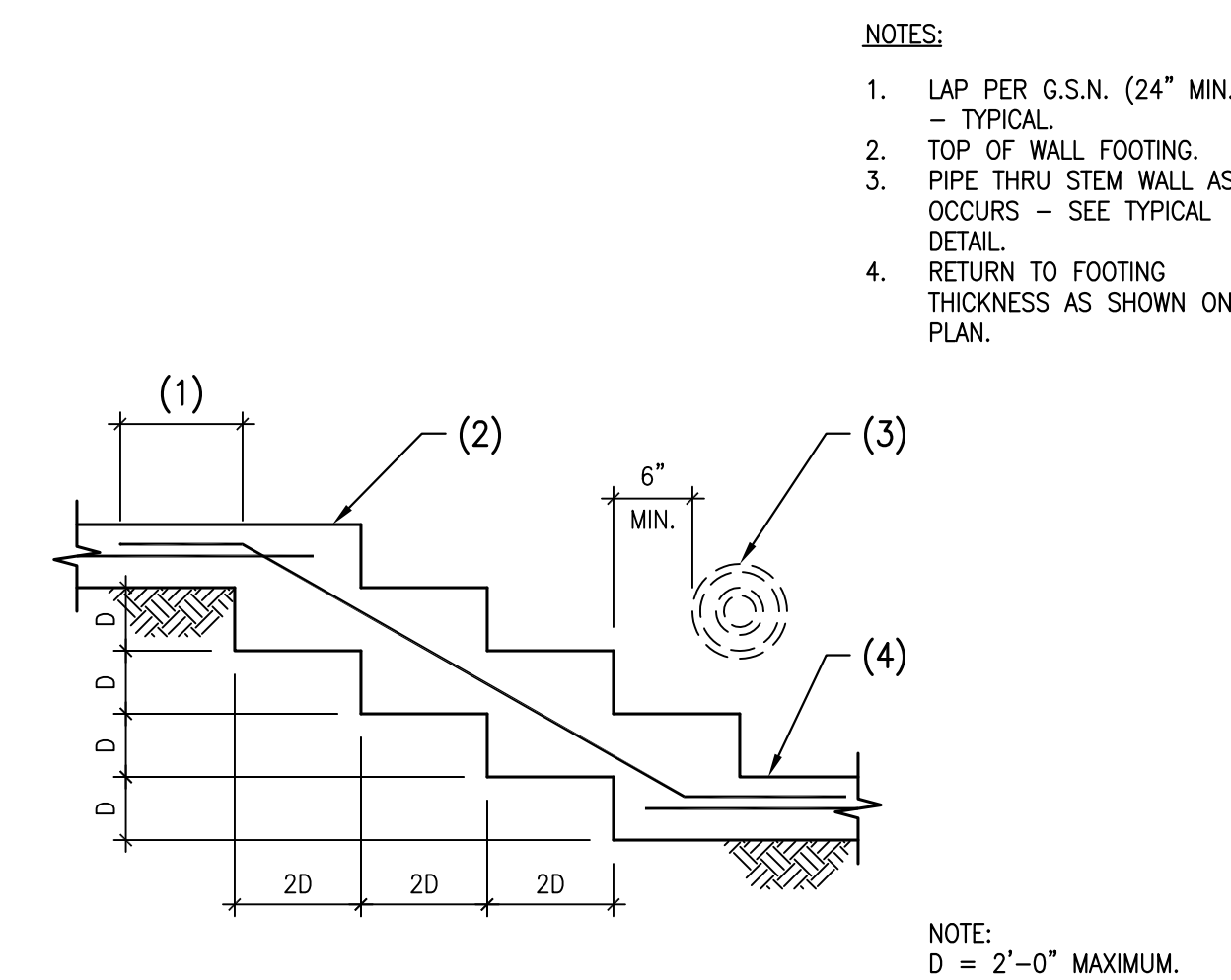
ANCHOR DIAMETER	VERT BOLT EMBEDMENT LENGTH	HORIZ BOLT EMBEDMENT LENGTH	HEADED STUD FILLET WELD SIZE, "S"
1/2"	7"	4"	1/4"
5/8"	7"	4"	5/16"
3/4"	7"	5"	5/16"
7/8"	8"	6"	5/16"
1"	9"	7"	3/8"
1 1/8"	10"	8"	---
1 1/4"	11"	9"	---



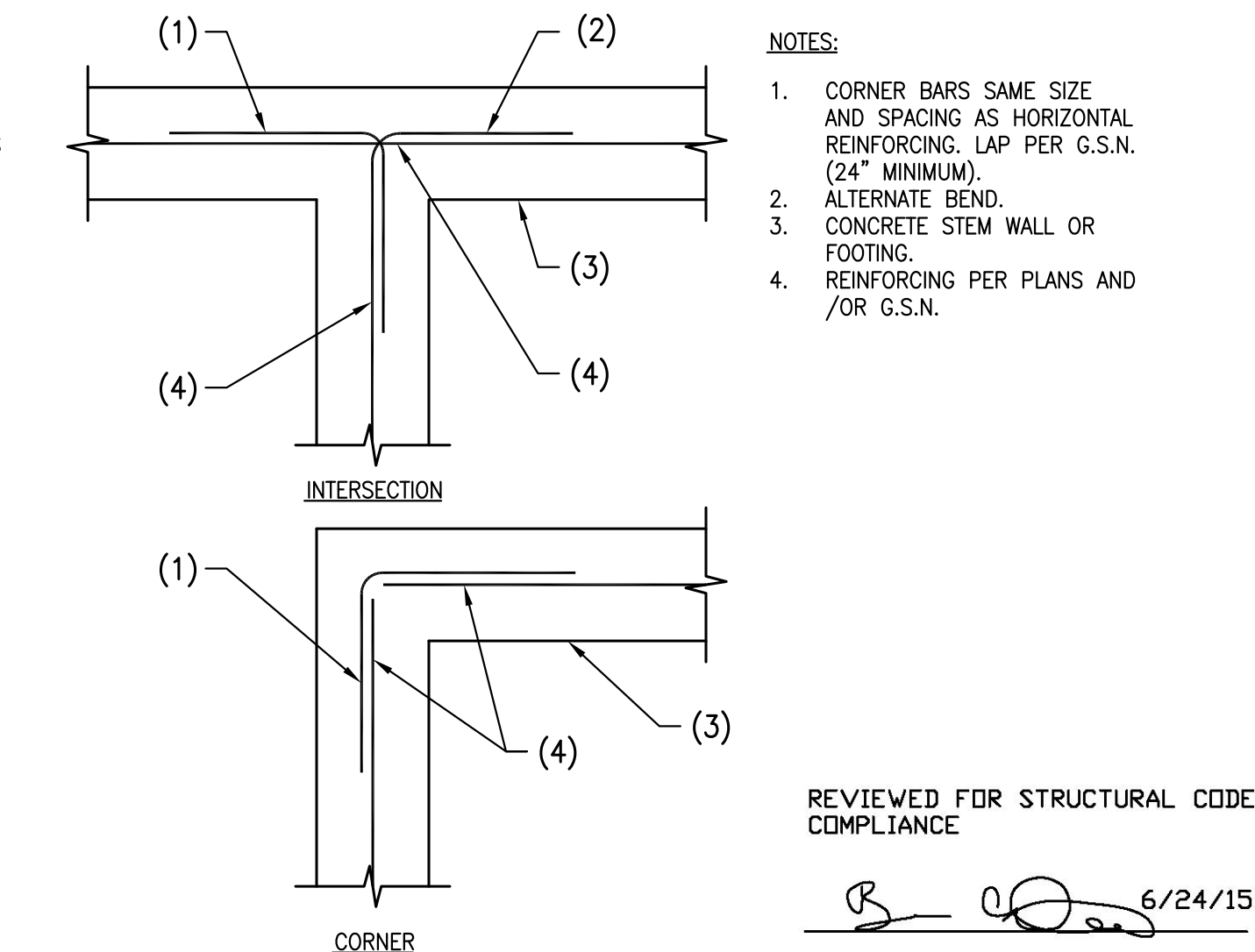
08 TYPICAL CAST-IN-PLACE ANCHOR, ANCHOR BOLT, AND HEADED STUD SCHEDULE
SCALE: NOT TO SCALE 501-001-TYP



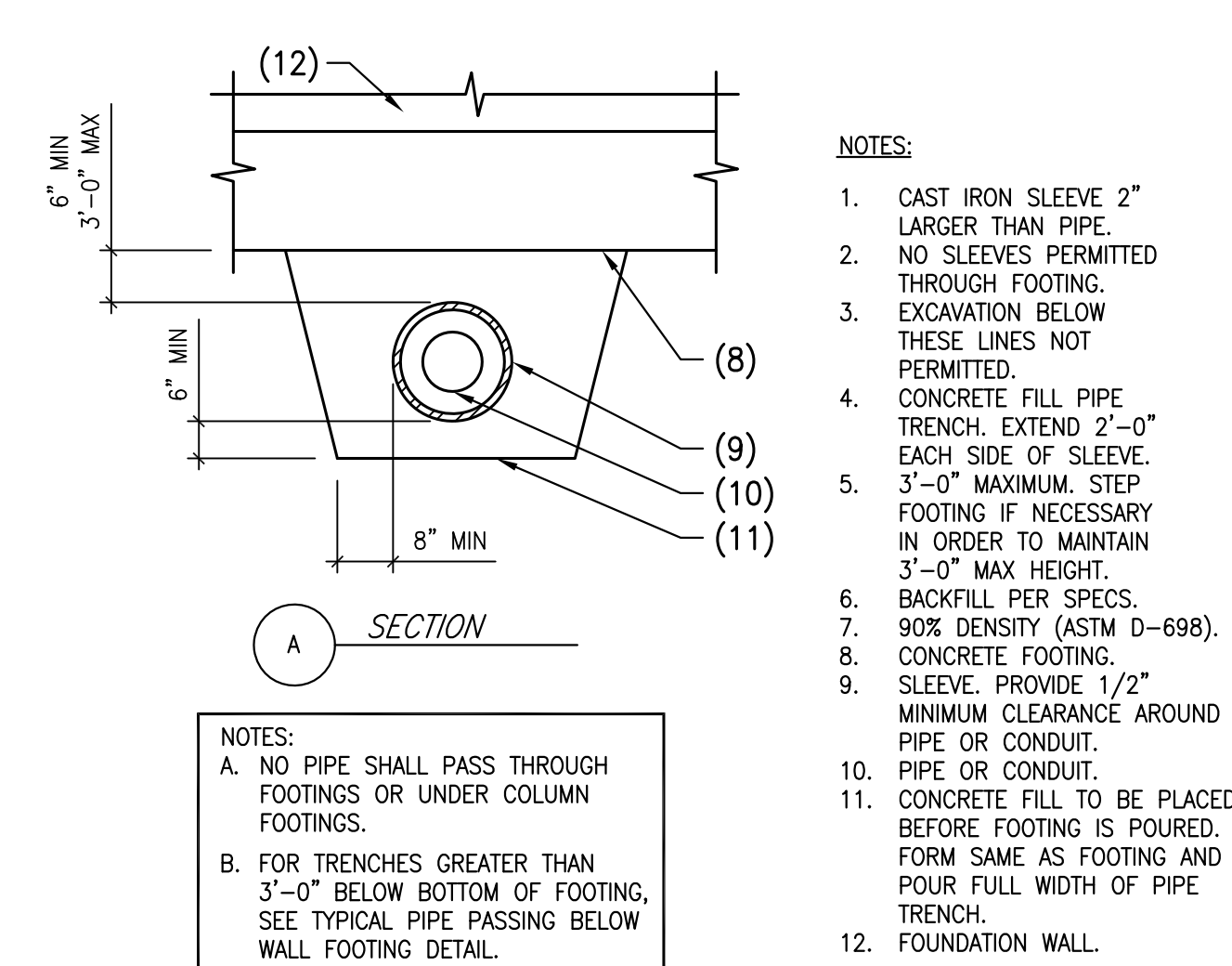
03 TYPICAL TRENCH PARALLEL TO FOUNDATION
SCALE: NOT TO SCALE 200-006-TYP



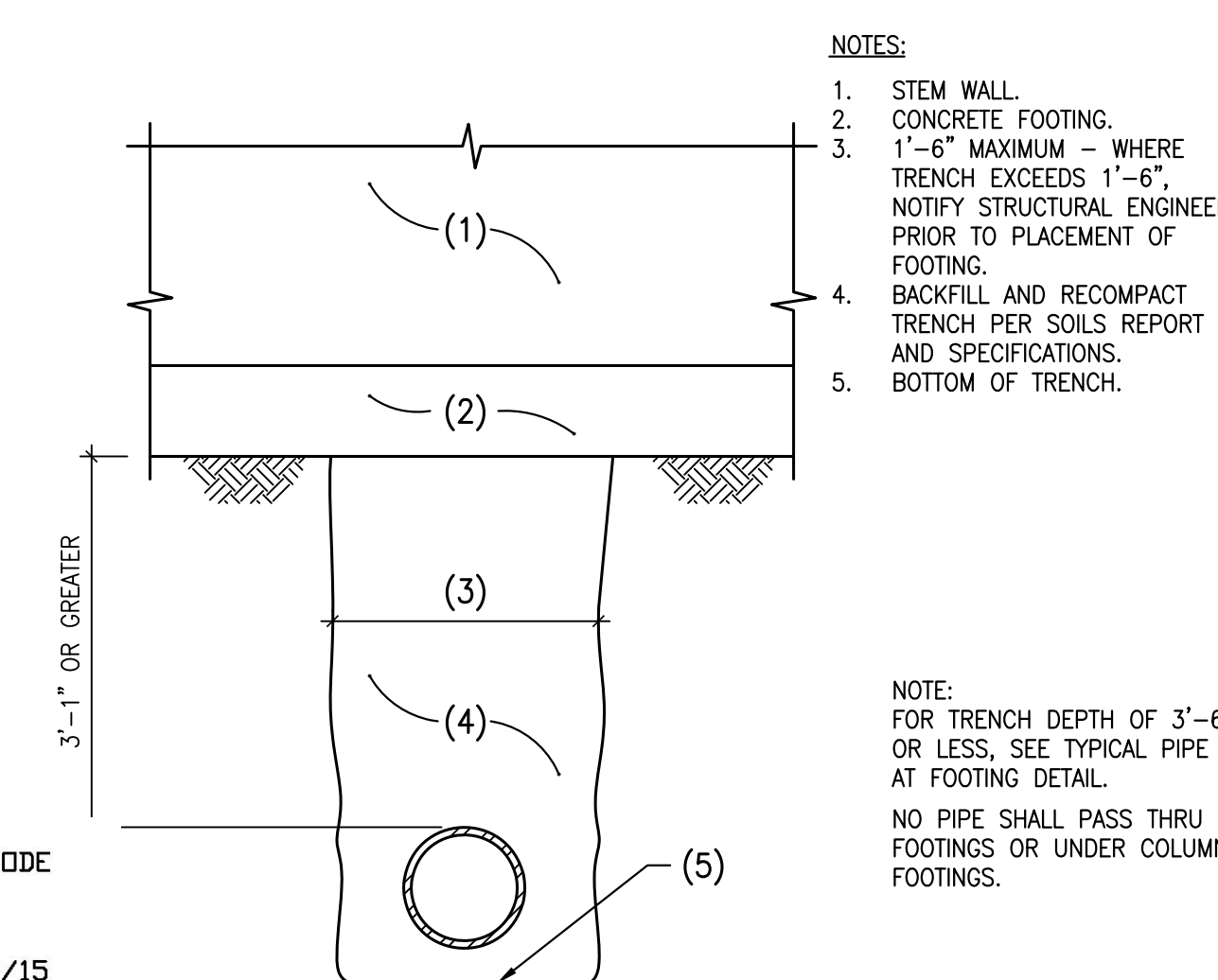
04 TYPICAL STEP IN CONCRETE FOOTING
SCALE: NOT TO SCALE 200-020-TYP



05 TYPICAL CORNER REINFORCING IN CONCRETE FOOTING AND/OR STEM WALL
SCALE: NOT TO SCALE 200-030-TYP



01 TYPICAL PIPE THROUGH FOOTING AND TRENCH
SCALE: NOT TO SCALE 200-005-TYP

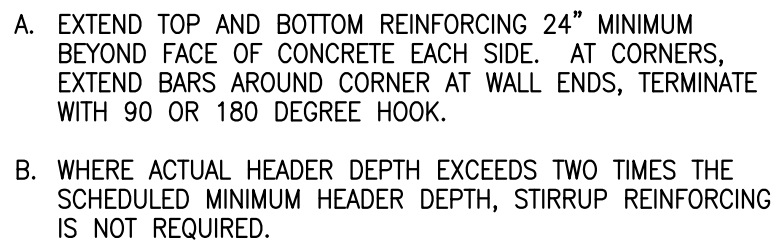


02 TYPICAL PIPE PASSING BELOW WALL FOOTING IN DEEP TRENCH
SCALE: NOT TO SCALE 200-007-TYP

revisions	number	date	comment

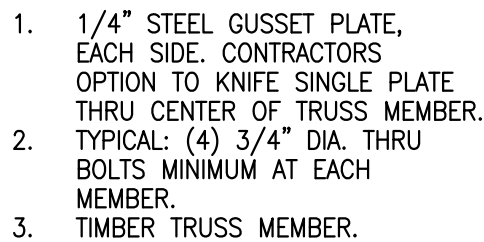
project	215078
engineer	W. OVIATT
drafter	R. BRANCH
date	06/17/15

1. I.C.F. WALL.
2. (2) #5 VERTICAL JAMB REINFORCING U.N.O.
3. TOP OF WALL OR COLD JOINT.
4. BOTTOM REINFORCING.
5. TOP REINFORCING.
6. SHEAR REINFORCING WITH 180 DEGREE HOOK EACH END. START SHEAR REINFORCING AT FACE OF ROUGH OPENING AT EACH JAMB.
7. FACE OF CONCRETE.
8. 2x LUMBER BUCK.



NOTE:
THIS SCHEDULE MAY BE USED ONLY IN WALLS WHERE NO OTHER HEADER HAS
BEEN CALLED OUT ON PLANS OR SCHEDULES.

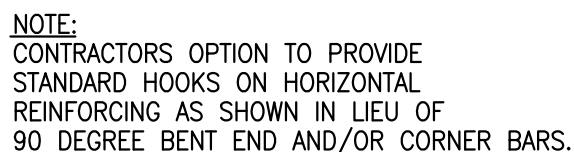
215078-S1.0-18



215078-S1.0-19

NOTE:
GEOMETRY IS GENERAL
REPRESENTATION. PLATE
AND MEMBER ANGLES
MAY VARY.

1. CONCRETE WALL.
2. HORIZONTAL REINFORCING.
3. VERTICAL REINFORCING PER
DRAWINGS AND AS SHOWN.
PLACE ON OUTSIDE OF DOUBLE
MAT.
4. END BAR TO MATCH AND LAP
HORIZONTAL REINFORCING.
5. CORNER BAR TO MATCH AND LAP
GREATER OF HORIZONTAL REINFORCING.
6. "U" BAR TO MATCH AND LAP
HORIZONTAL REINFORCING.
7. SPLICE/LAP PER G.S.N. AND/OR
TYPICAL DETAILS.
8. STAGGER SPLICES.
9. 90 DEGREE OR 180 DEGREE HOOK
PER PLANS AND/OR G.S.N. AT
EACH END BAR.



215078-S1.0-16

1. EDGE ATTACHMENT.
2. WOOD TRUSS.
NOTE: TRUSS MANUFACTURER
TO PROVIDE VERTICAL WEB
TO MATCH LOCATION OF SHEAR
PANEL.
3. 7/16" PLYWOOD SHEAR PANEL
WITH 8d AT 6" O.C. AT PANEL
EDGES. ORIENT FACE GRAIN
VERTICALLY.
4. 16d AT 6" O.C. TO TRUSS
VERTICAL WEB.
5. 16d TO MATCH "EDGE ATTACHMENT"
SPACING.
6. PLYWOOD SHEATHING.
2x4 AT FOUR SPACES.
8. TOP OF WOOD PLATE OR
BEAM OCCURS
9. DRILLED VENTILATION HOLES PER
ARCHITECTURAL.

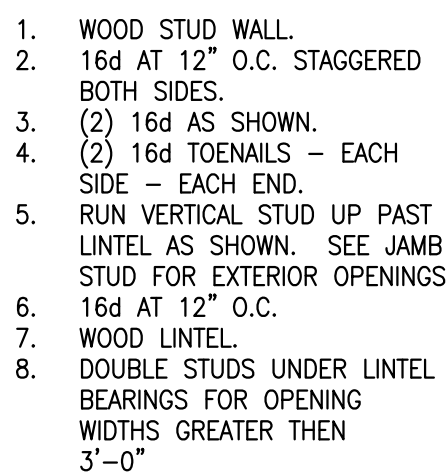


215078-S1.0-17



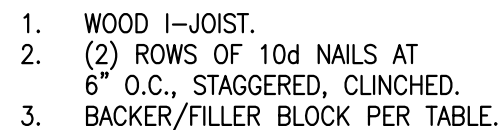
SCALE: NOT TO SCALE

601-007-TYP



NOTE:
THIS SCHEDULE MAY BE
USED ONLY IN WALLS WHERE
NO OTHER LINTEL HAS BEEN
CALLED OUT ON THE PLANS
OR SCHEDULES.

601-002-TYPE



NOTE:
CUT BACKER AND FILLER BLOCKS TO A MAXIMUM DEPTH
EQUAL TO THE WEB DEPTH MINUS 1/4" TO AVOID A
FORCED FIT. EXTEND WEB FILLER 4" PAST.

SCALE: NOT TO SCALE

215078-S1.0-15

Re 00 6/24/15

NOTES:

- 1.COMMON OR BOX NAILS ARE PERMITTED TO BE USED EXCEPT WHERE OTHERWISE STATED.
- 2.STAPLES SHALL HAVE A MINIMUM CROWN OF 7/16".
- 3.FASTENING SCHEDULE APPLIES U.N.O., WHERE DIFFERENCES OCCUR, GREATER REQUIREMENT GOVERNS.

601-008-TYP



SCALE: NOT TO SCALE

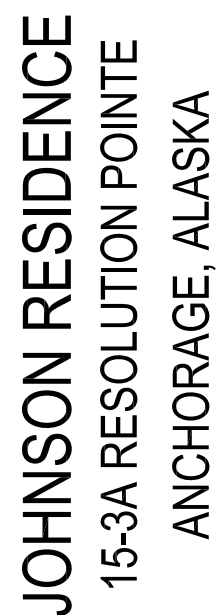
601-006-TYF

1. DRILLED HOLE DIAMETER. NOT TO EXCEED 1/3 OF THE JOIST/BEAM DEPTH.
2. WOOD JOIST OR BEAM.
3. DRILLED HOLES ARE PERMITTED ONLY IN HATCHED AREA.
4. WOOD STUD.
5. DRILLED HOLE DIAMETER. NOT TO EXCEED 40 PERCENT OF STUD WIDTH (SEE NOTE).

A. DRILLED HOLES IN WOOD STUDS MAY BE INCREASED TO 60 PERCENT OF THE STUD WIDTH, PROVIDED THAT (1) THE WALL IS A NON-BEARING WALL SUPPORTING NO LOADS OTHER THAN THE WEIGHT OF THE WALL OR (2) THE STUDS ARE DOUBLED (NO MORE THAN TWO ADJACENT DOUBLED/DRILLED STUDS ARE PERMITTED).

B. THIS DETAIL APPLIES TO SOLID SAWN LUMBER ONLY, NOT TO ENGINEERED WOOD PRODUCTS OR GLULAMS.

C. CONTACT ENGINEER IF CONDITIONS EXCEED THOSE SHOWN.



TYPICAL DETAILS



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project 215078

engineer W. OVIATT

drafter R. BRANCH

date 06/17/15

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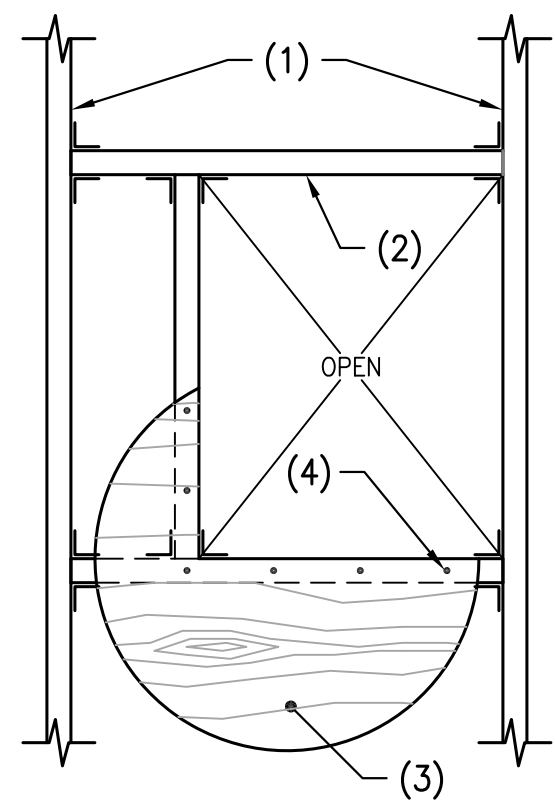
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NOTES:

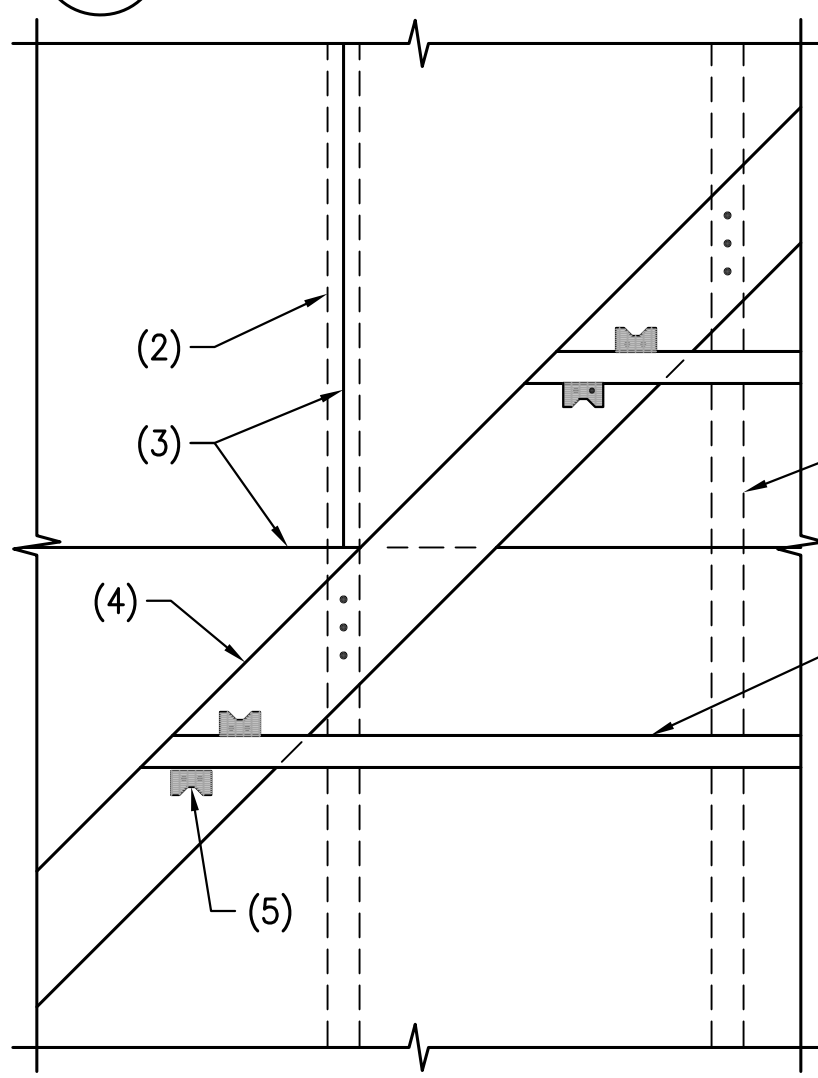
1. WOOD TRUSS OR 1-JOIST.
2. TYPICAL: 2x6 WITH SIMPSON LUS26 HANGER OR (2) SIMPSON A35 CLIPS.
3. PLYWOOD ROOF SHEATHING.
4. RE-ATTACH EXISTING SHEATHING WITH 10d NAILS AT 6" O.C.



20 TYPICAL FRAMED OPENING IN PLYWOOD ROOF/FLOOR SHEATHING
SCALE: NOT TO SCALE 215078-S1.0-20

NOTES:

1. 2x6 AT 24" O.C., MAX SPAN = 6'-0".
2. WOOD TRUSS BELOW.
3. LINE OF SHEATHING JOINTS.
4. 2x8 LAID FLAT OVER SHEATHING WITH (3) 10d AT EACH TRUSS.
5. SIMPSON A34 EACH SIDE - TYPICAL.



NOTES:

- A. PLYWOOD SHEATHING OVER PRIMARY ROOF TRUSSES SHALL BE CONTINUOUS.
- B. FOR CLARITY, SHEATHING OVER VALLEY FRAMING NOT SHOWN.

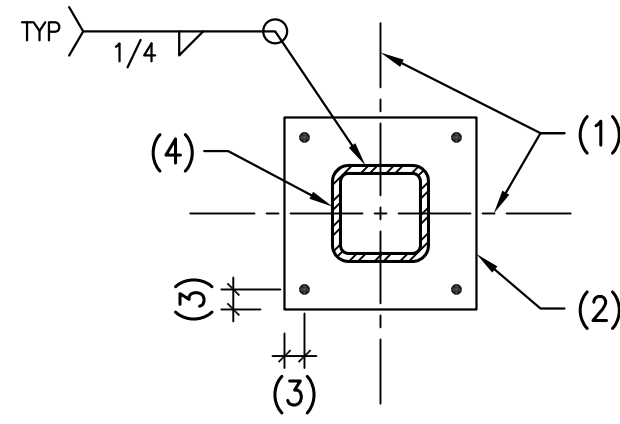
21 PLAN - TYPICAL VALLEY OVERFRAMING AT WOOD TRUSSES
SCALE: NOT TO SCALE 215078-S1.0-21

NOTES:

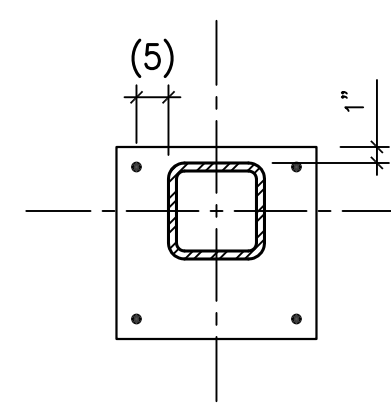
1. CENTERLINE OF COLUMN AND STEEL BASE PLATE.
2. STEEL BASE PLATE.
3. 1 1/4" MINIMUM OR PER A.I.S.C. TABLE U3.4.
4. STEEL COLUMN - FOR TYPE, SIZE, BASE PLATE AND ANCHOR BOLTS, SEE SCHEDULE.
5. 2" MINIMUM OR AS REQUIRED FOR WRENCHING CLEARANCE.

NOTE:

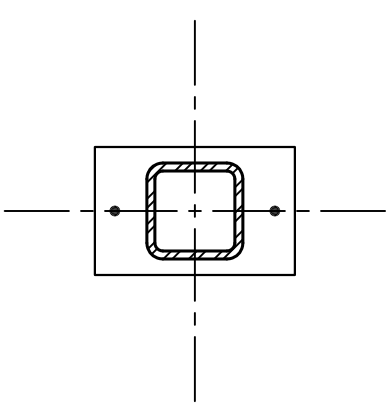
PROVIDE TEMPORARY SUPPORT DURING ERECTION FOR COLUMNS WITH 2 BOLT BASE PLATES.



TYPICAL BASE PLATE



OFFSET BASE PLATE



2 BOLT BASE PLATE

22 TYPICAL STEEL COLUMN BASE PLATE
SCALE: NOT TO SCALE 505-003-TYP

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6/24/15

WALL OPENING (WO) SCHEDULE				
NOTES: A. EXTEND TOP AND BOTTOM REINFORCING 24" MINIMUM BEYOND FACE OF CONCRETE EACH SIDE. AT CORNERS, EXTEND BARS AROUND CORNER AT WALL ENDS, TERMINATE WITH 90 OR 180 DEGREE HOOK. B. WHERE ACTUAL HEADER DEPTH EXCEEDS TWO TIMES THE SCHEDULED MINIMUM HEADER DEPTH, STIRRUP REINFORCING IS NOT REQUIRED.				
MARK	ROUGH OPENING WIDTH	MINIMUM HEADER DEPTH	TOP AND BOTTOM REINFORCING	STIRRUP REINFORCING
WO1	UP TO 3'-0"	10"	(1) #5 T & B	---
WO2	3'-1" TO 8'-0"	10"	(2) #5 T & B	(2) #3 AT 8" O.C.
WO3	8'-1" TO 13'-6"	12"	(2) #5 TOP (2) #7 BOTTOM	(2) #3 AT 8" O.C.
NOTES: 1. I.C.F. WALL. 2. TYPICAL ALL OPENINGS: HALF OF WALL REINFORCEMENT REMOVED AT OPENING TO BE PLACED EACH SIDE OF OPENING. USE (2) #5 BARS MINIMUM EACH SIDE TOP AND BOTTOM. 3. TOP OF WALL OR COLD JOINT. 4. BOTTOM REINFORCING. 5. TOP REINFORCING. 6. SHEAR REINFORCING WITH 180 DEGREE HOOK EACH END. START SHEAR REINFORCING AT FACE OF ROUGH OPENING AT EACH JAMB. 7. FACE OF CONCRETE. 8. TREATED 2x LUMBER BUCK.				

LEDGER (L) SCHEDULE				
MARK	TYPE	SIZE	CONNECTION	REMARKS
L1	WOOD	2x12	SIMPSON ICVL WITH ICVL-CW AT 48" O.C. WITH (8) ICF-D3.62 SCREWS	---
L2	WOOD	2x8	SIMPSON ICVL WITH ICVL-CW AT 48" O.C. WITH (8) ICF-D3.62 SCREWS	---
NOTES: 1. ALL LEDGERS SHALL HAVE MINIMUM OF 2 ANCHORS/FASTENERS PER LEDGER PIECE. 2. ANCHORS/FASTENERS SHALL BE LOCATED NOT LESS THAN 6" NOR MORE THAN 12" FROM END OF LEDGER PIECES OR AT LEDGER SPLICES.				

JOIST (J) SCHEDULE				
MARK	TYPE/SIZE	JOIST SPACING	END CONNECTION	REMARKS
J1	11 7/8" BCI 6000	16" O.C.	SIMPSON IUS OR ITS HANGER	---
J2	2x10	16" O.C.	SIMPSON LUS HANGER	---

WOOD TRUSS (T) SCHEDULE			
MARK	CONFIGURATION	SPACING	REMARKS
T1		24" O.C. U.N.O.	SEE NOTE 1
T2		24" O.C. U.N.O.	SEE NOTE 1
T3		24" O.C. U.N.O.	SEE NOTE 1
T4		24" O.C. U.N.O.	SEE NOTE 1
T5		24" O.C. U.N.O.	SEE NOTE 1
T6		24" O.C. U.N.O.	SEE NOTE 1
T7		24" O.C. U.N.O.	SEE NOTE 1
T8		24" O.C. U.N.O.	SEE NOTE 1
T9		24" O.C. U.N.O.	SEE NOTE 1
NOTES: 1. SEE ARCHITECTURAL DRAWINGS FOR REQUIRED DIMENSIONS, DEPTHS, PROFILES, ETC. 2. TRUSS FABRICATOR TO PROVIDE OVERFRAMING AT AREAS SHOWN ON PLANS. AT FABRICATORS OPTION, TRUSSES MAY BE FABRICATED TO INCLUDE OVERFRAMING (PIGGY BACK) INTO TRUSS CONFIGURATION. 3. TRUSS GIRDER DESIGNATED AS TG#. 4. TRUSSES SHALL BE DESIGNED FOR AN ADDITIONAL 300 LBS CONCENTRATED DEAD LOAD AT ANY POINT.			

FOOTING (F) SCHEDULE					
<div><p>FOR CONSTRUCTION ABOVE FOOTING, SEE DETAILS</p><p>TOP REINFORCING WHERE INDICATED ON SCHEDULE</p><p>FOOTING REINFORCING</p><p>2" CLR</p><p>3" CLR</p><p>3" CLR</p><p>HEIGHT</p><p>WIDTH</p></div>					
MARK	DIMENSIONS			FOOTING REINFORCING	REMARKS
	HEIGHT	WIDTH	LENGTH		
F1	10"	2'-0"	CONT.	(3) #5 BARS CONT.	---
F2	10"	1'-8"	CONT.	(3) #5 BARS CONT.	---
F3	10"	1'-4"	CONT.	(2) #5 BARS CONT.	---
F4	10"	3'-0"	3'-0"	(4) #5 BARS, EACH WAY	---
F5	10"	4'-0"	4'-0"	(5) #5 BARS, EACH WAY	---
F6	12"	6'-0"	6'-0"	(7) #5 BARS, EACH WAY	---
NOTES: 1. FOR FOOTING BEARING DEPTH BELOW GRADE, SEE G.S.N. U.N.O. 2. CENTER FOOTINGS UNDER WALLS OR COLUMNS U.N.O.					

COLUMN (C) SCHEDULE			
MARK	SIZE	BASE CONNECTION	REMARKS
C1	6x6	SIMPSON ABU66 U.N.O.	---
C2	3" XXSTRONG STEEL PIPE	(4) 5/8" DIA. C.I.P. ANCHOR BOLTS SEE TYPICAL DETAILS FOR EMBEDMENT AND BASE PLATE GEOMETRY	---

BEAM (B) SCHEDULE			
MARK	SIZE	END CONNECTION	REMARKS
B1	4x12	PER PLAN/DETAILS	---
B2	5 1/8" x 12" GLB	PER PLAN/DETAILS	---
B3	5 1/8" x 16 1/2" GLB	PER PLAN/DETAILS	---
B4	8 3/4" x 16 1/2" GLB	PER PLAN/DETAILS	---

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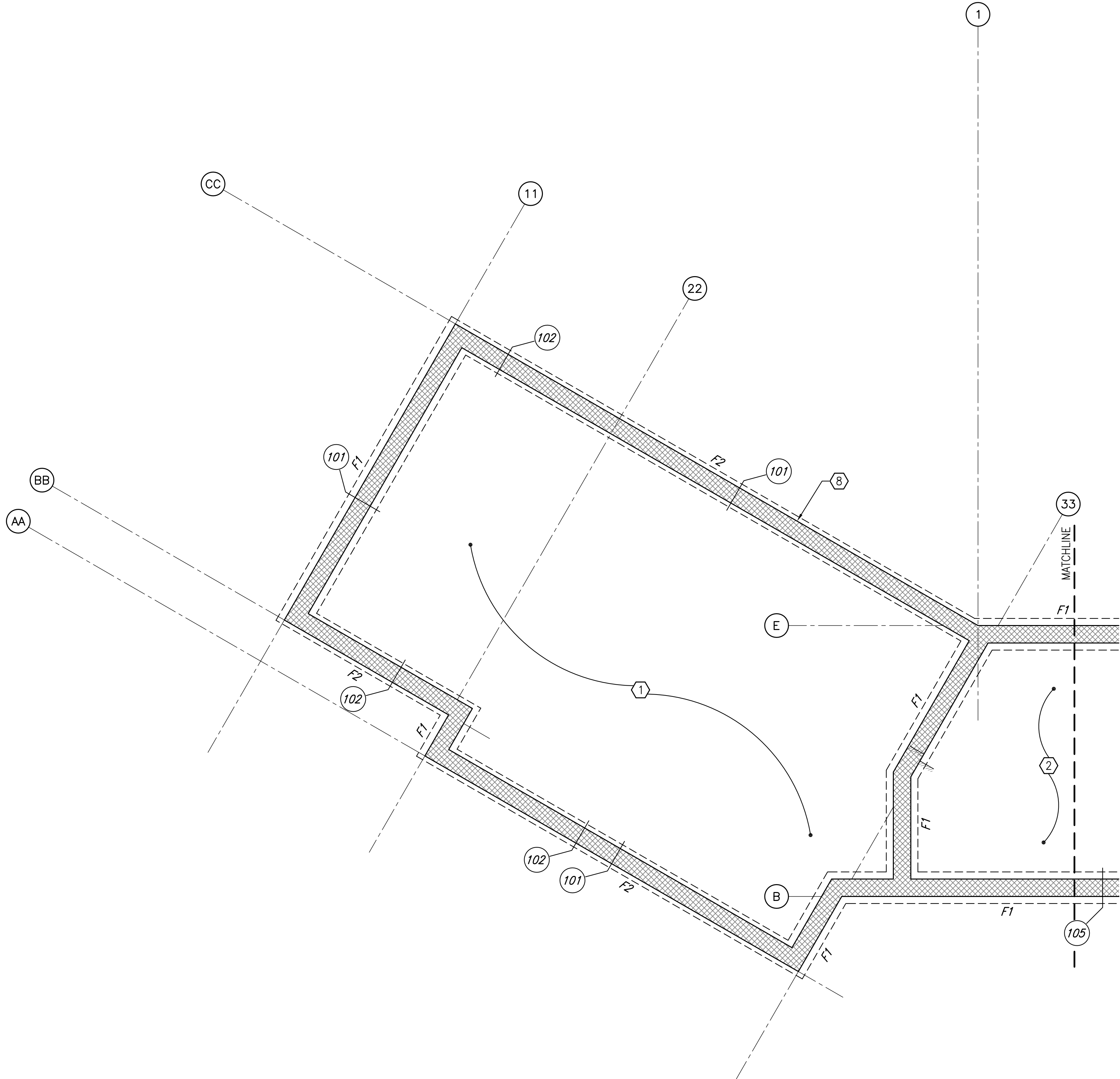
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drafter	R. BRANCH
date	06/17/15



FOUNDATION PLAN

SCALE: 1/4"=1'-0"

FOUNDATION KEYNOTES:

- ① UNEXCAVATED NATIVE SOIL OR FILL.
- ② HEATED CRAWL SPACE.
- ③ TYPICAL AS INDICATED: WOOD COLUMN.
- ④ WOOD STUD PONY WALL.
- ⑤ TYPICAL AS INDICATED: 3" DIA. MINI PILE, PER MOA POLICY. DESIGN LOAD = 4.9 KIPS.
- ⑥ UNHEATED CRAWL SPACE.
- ⑦ STEP FOOTING AS REQUIRED TO MAINTAIN FROST PROTECTION PER G.S.N.
- ⑧ TYPICAL U.N.O.: 8" I.C.F. CONCRETE WALL, REINFORCE WITH #5 VERTICALS AT 16" O.C. AND #5 HORIZONTALS AT 16" O.C., CENTERED IN WALL. SEE "TYPICAL REINFORCING IN CONCRETE WALL" DETAIL. AT WALL ENDS AND OPENINGS, INSTALL 180 DEGREE HOOK. AT CORNERS, INSTALL 90 DEGREE REINFORCING.

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B. Oviatt 6/24/15

JOHNSON RESIDENCE
15-3A RESOLUTION POINTE
ANCHORAGE, ALASKA
FOUNDATION PLAN

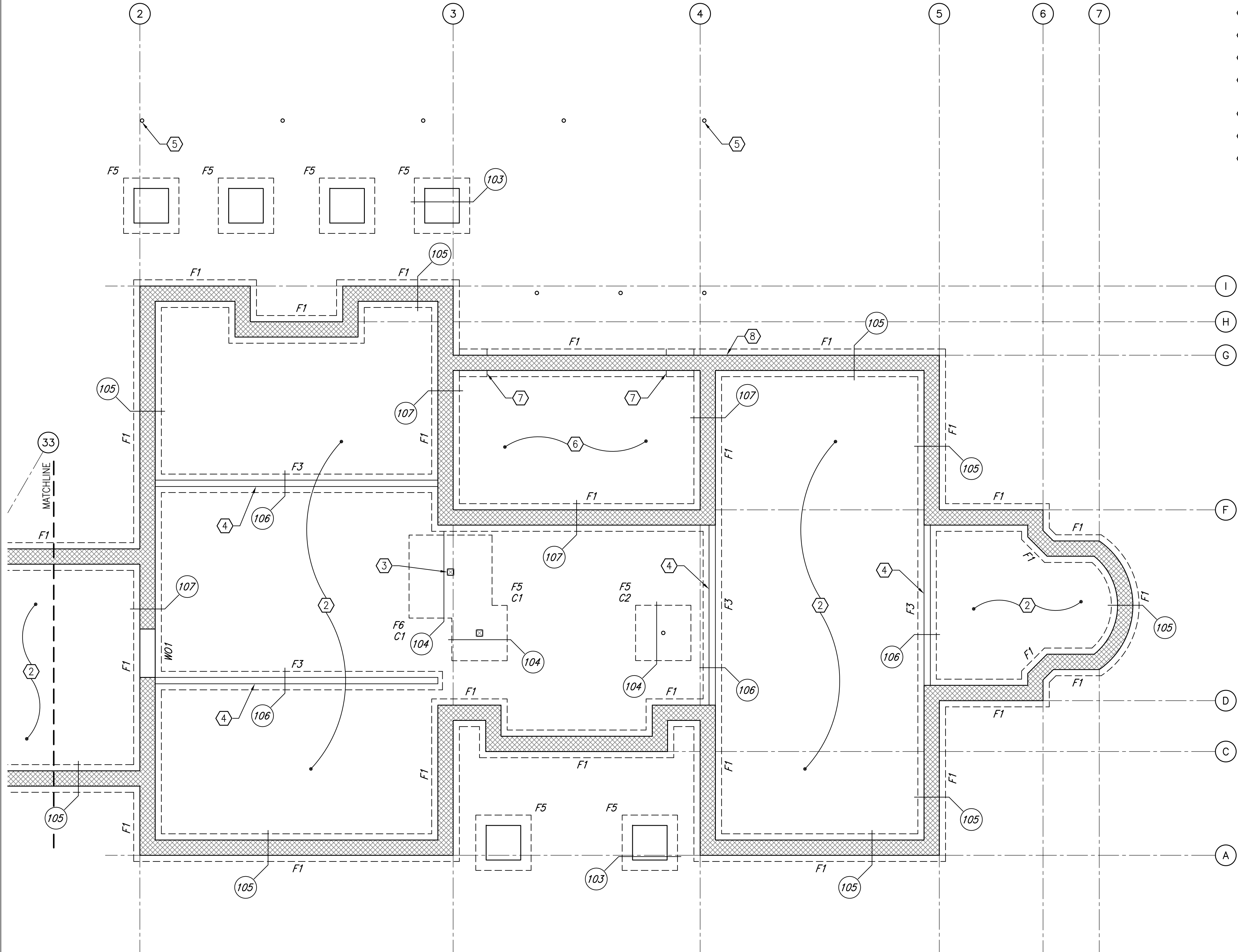


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number	date	comment

project 215078
engineer W. OVIATT
drafter R. BRANCH
date 06/17/15

S2.0
sheet



FOUNDATION KEYNOTES:

- ① UNEXCAVATED NATIVE SOIL OR FILL.
- ② HEATED CRAWL SPACE.
- ③ TYPICAL AS INDICATED: WOOD COLUMN.
- ④ WOOD STUD PONY WALL.
- ⑤ TYPICAL AS INDICATED: 3" DIA. MINI PILE, PER MOA POLICY. DESIGN LOAD = 4.9 KIPS.
- ⑥ UNHEATED CRAWL SPACE.
- ⑦ STEP FOOTING AS REQUIRED TO MAINTAIN FROST PROTECTION PER G.S.N.
- ⑧ TYPICAL U.N.O.: 8" I.C.F. CONCRETE WALL, REINFORCE WITH #5 VERTICALS AT 16" O.C. AND #5 HORIZONTALS AT 16" O.C., CENTERED IN WALL. SEE "TYPICAL REINFORCING IN CONCRETE WALL" DETAIL. AT WALL ENDS AND OPENINGS, INSTALL 180 DEGREE HOOK. AT CORNERS, INSTALL 90 DEGREE REINFORCING.

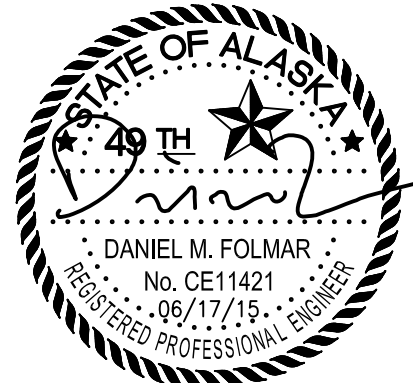
FOUNDATION PLAN

SCALE: 1/4"=1'-0"

REVIEWED FOR STRUCTURAL CODE COMPLIANCE

[Signature] 6/24/15

JOHNSON RESIDENCE
15-3A RESOLUTION POINTE
ANCHORAGE, ALASKA
FOUNDATION PLAN

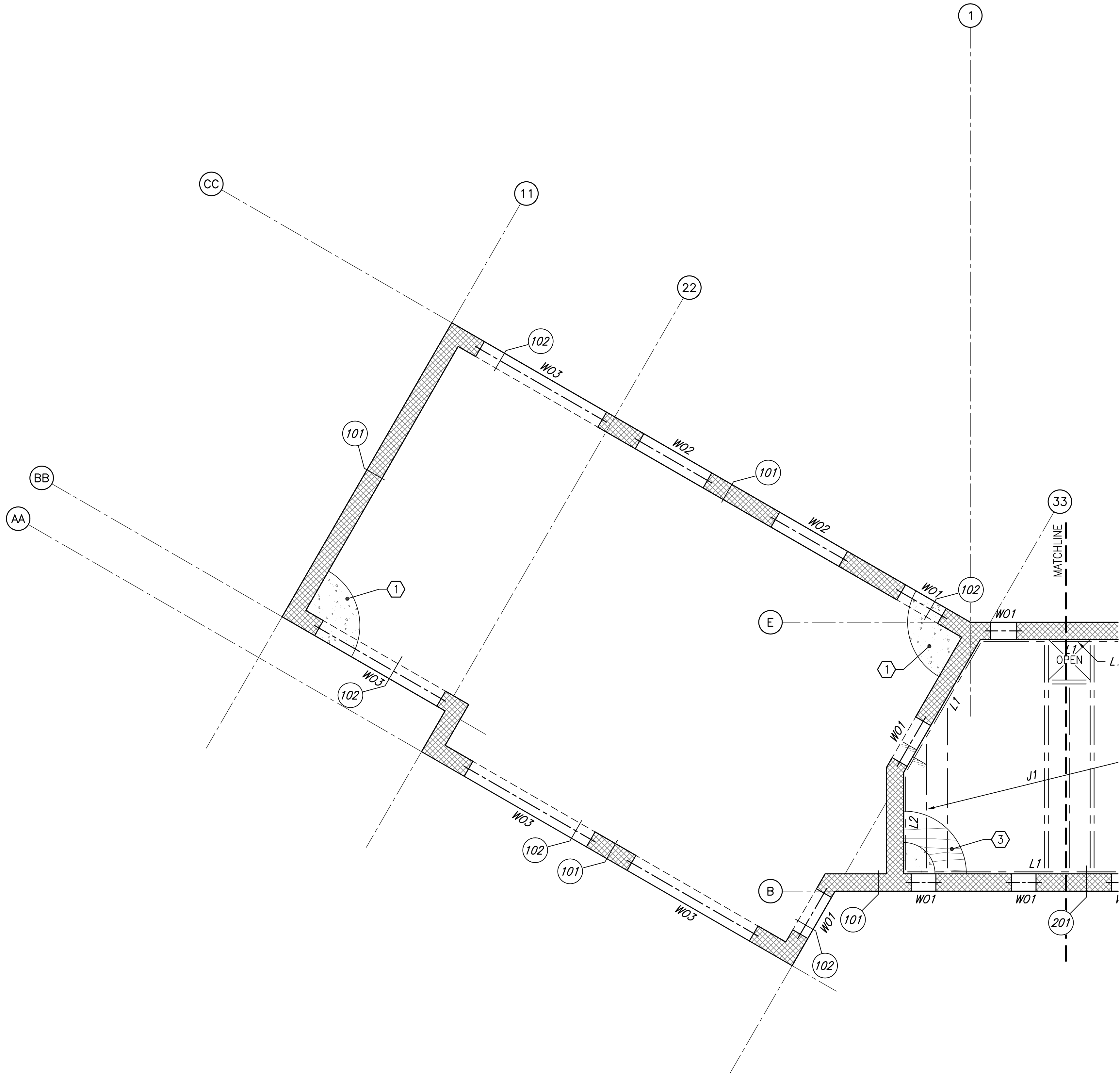


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revisions		
number	date	comment

project	215078
engineer	W. OVIATT
drafter	R. BRANCH
date	06/17/15

S2.1
sheet



FIRST FLOOR FRAMING PLAN
SCALE: 1/4"=1'-0"

- FRAMING KEYNOTES:**
- ① 4" CONCRETE SLAB ON GRADE. REINFORCE WITH 6x6 W1.4-W1.4 W.W.F. 1 1/2" CLEAR FROM TOP OF SLAB.
 - ② WOOD STAIRS PER G.S.N.
 - ③ 1 1/2" GYPCRETE OVER FLOOR SHEATHING PER G.S.N.
 - ④ SIMPSON HGU BEAM HANGER.
 - ⑤ TYPICAL: SIMPSON CC TYPE WELDED COLUMN CAP AT BEAMS CONNECTION TO MINI PILES.
 - ⑥ TYPICAL: SIMPSON ECC TYPE WELDED COLUMN CAP AT END BEAMS CONNECTING TO MINI PILES.
 - ⑦ DOUBLE FLOOR JOISTS AT STAIR LANDING.
 - ⑧ WOOD DECKING PER ARCHITECTURAL.
 - ⑨ TYPICAL: FRAME COLUMN CONTINUOUS THROUGH FLOOR. PROVIDE SUPPORT AROUND OPENING PER TYPICAL DETAIL. FRAME TIGHT AROUND COLUMN, ALL SIDES.

REVIEWED FOR STRUCTURAL CODE COMPLIANCE
 6/24/15

JOHNSON RESIDENCE
15-3A RESOLUTION POINTE
ANCHORAGE, ALASKA
FIRST FLOOR FRAMING PLAN



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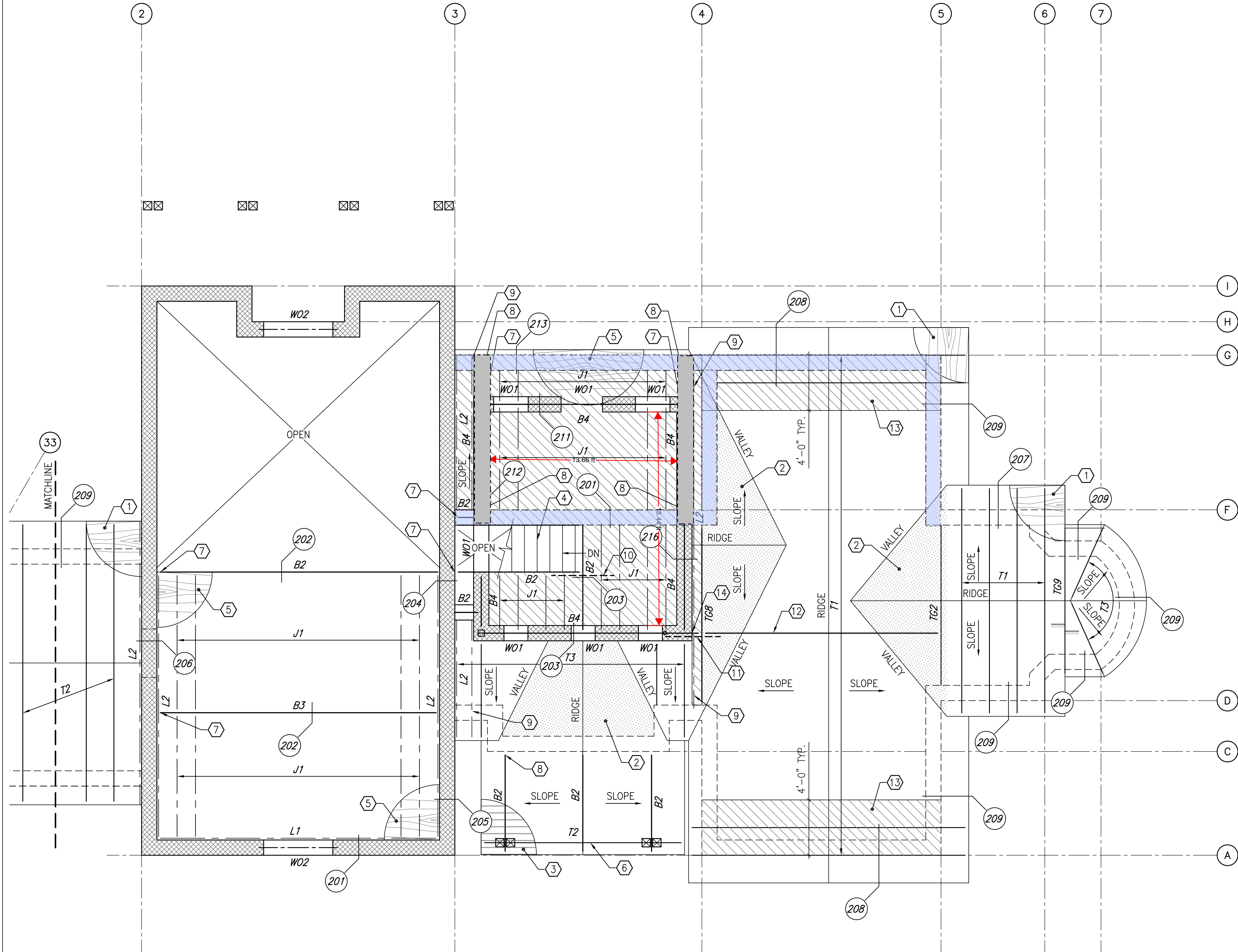
revisions		
number	date	comment

project	215078
engineer	W. OVIATT
drafter	R. BRANCH
date	06/17/15

S2.2
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S2.4
sheet

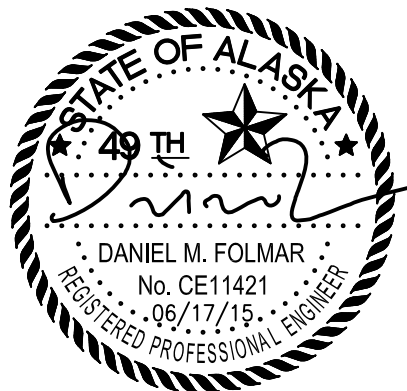


- FRAMING KEYNOTES:**
- 1 ROOF SHEATHING PER G.S.N.
 - 2 SHADED AREA INDICATES OVERFRAMING PER TYPICAL DETAIL.
 - 3 3" WOOD ROOF DECKING PER G.S.N.
 - 4 WOOD STAIRS PER G.S.N.
 - 5 FLOOR SHEATHING PER G.S.N.
 - 6 ARCHITECTURALLY EXPOSED TIMBER END TRUSS. SEE TYPICAL DETAIL FOR CONNECTION INFORMATION.
 - 7 SIMPSON HGU BEAM HANGER.
 - 8 POCKET BEAM INTO I.C.F. WALL. PROVIDE ROOFING FELT PROTECTION FOR BEAM PER G.S.N.
 - 9 EXTEND WOOD LEDGER TO BEAR ON I.C.F. WALL. NO SPLICE IN LEDGER WITHIN 10'-0" OF WALL CORNER.
 - 10 SIMPSON CS14 STRAP. LAP 16" ONTO BEAM, AND LAP OTHER END 4'-0" ONTO FLAT 2x BLOCKING BETWEEN JOIST BAYS. FASTEN WITH 10d NAILS.
 - 11 SIMPSON CS14 STRAP. LAP EACH END 16" ONTO BOTTOM OF BEAM/TRUSS FASTEN WITH 10d NAILS.
 - 12 ALIGN A TRUSS WITH BEAM BEYOND.
 - 13 CROSS HATCHING INDICATES FLAT 2x BLOCKING. BLOCKING IS REQUIRED AT ALL DIAPHRAGM PANEL EDGES AS INDICATED. PROVIDE EDGE ATTACHMENT PER G.S.N.
 - 14 EXTEND CANTILEVERED BEAM TIGHT TO T8, TO INSTALL STRAP WITH TRUSS BEYOND.

SECOND FLOOR AND LOW ROOF FRAMING PLAN
SCALE: 1/4"=1'-0"

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[Signature] 6/24/15

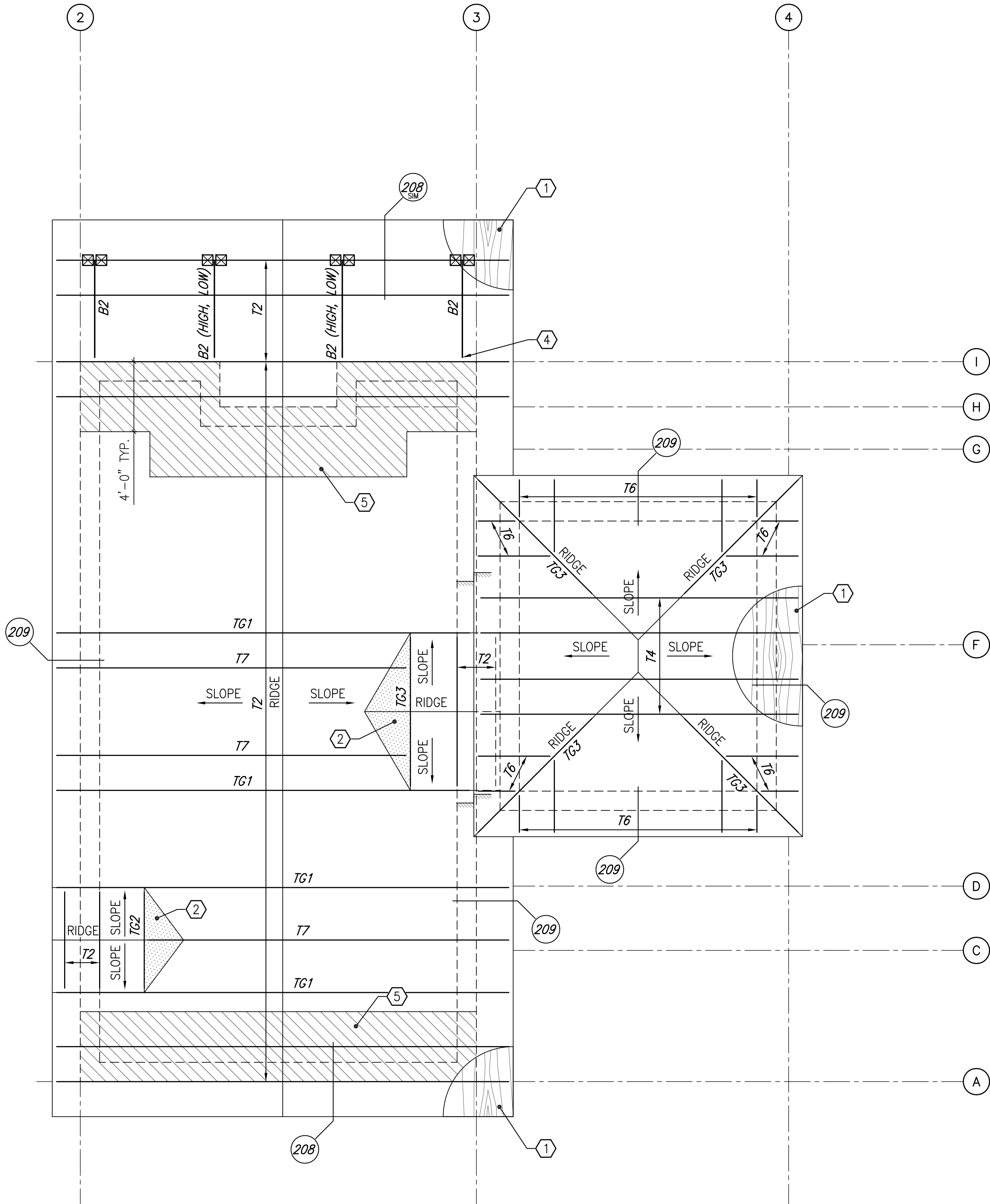
JOHNSON RESIDENCE
15-3A RESOLUTION POINTE
ANCHORAGE, ALASKA
SECOND FLOOR AND LOW ROOF FRAMING PLAN



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project 215078
engineer W. OVIATT
drafter R. BRANCH
date 06/17/15
S2.5
sheet



HIGH ROOF FRAMING PLAN
SCALE: 1/4"=1'-0"

- FRAMING KEYNOTES:**
- 1 ROOF SHEATHING PER G.S.N.
 - 2 SHADED AREA INDICATES OVERFRAMING PER TYPICAL DETAIL.
 - 3 ARCHITECTURALLY EXPOSED TIMBER END TRUSS. SEE TYPICAL DETAIL FOR CONNECTION INFORMATION.
 - 4 TYPICAL: POCKET BEAM INTO I.C.F. WALL. PROVIDE ROOFING FELT PROTECTION FOR BEAM PER G.S.N.
 - 5 CROSS HATCHING INDICATES FLAT 2x BLOCKING. BLOCKING IS REQUIRED AT ALL DIAPHRAGM PANEL EDGES AS INDICATED. PROVIDE EDGE ATTACHMENT PER G.S.N.

REVIEWED FOR STRUCTURAL CODE COMPLIANCE
[Signature] 6/24/15

JOHNSON RESIDENCE
15-3A RESOLUTION POINTE
ANCHORAGE, ALASKA
HIGH ROOF FRAMING PLAN



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project	215078
engineer	W. OVIATT
drafter	R. BRANCH
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- 215078-S3.0-107



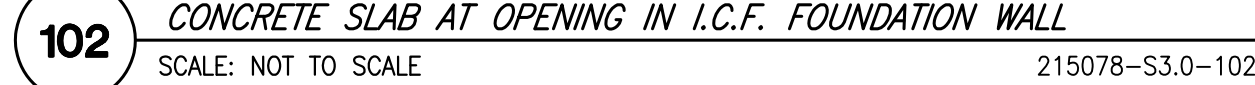
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- 215078-S3.0-105



- 215078-S3.0-102




- 215078-S3.0-103



- 215078-S3.0-106

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JOHNSON RESIDENCE
15-3A RESOLUTION POINTE
ANCHORAGE, ALASKA
FOUNDATION DETAILS

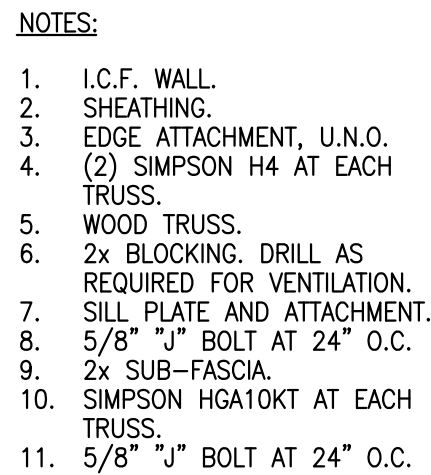


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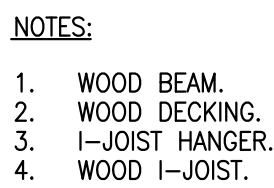
project	215078
engineer	W. OVIATT
draftee	R. BRANCH
date	06/17/15

S3.0
sheet



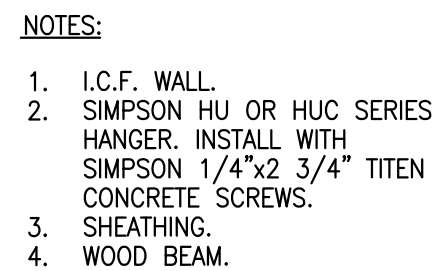
209 WOOD TRUSS AT I.C.F. WALL

215078-S4.0-209



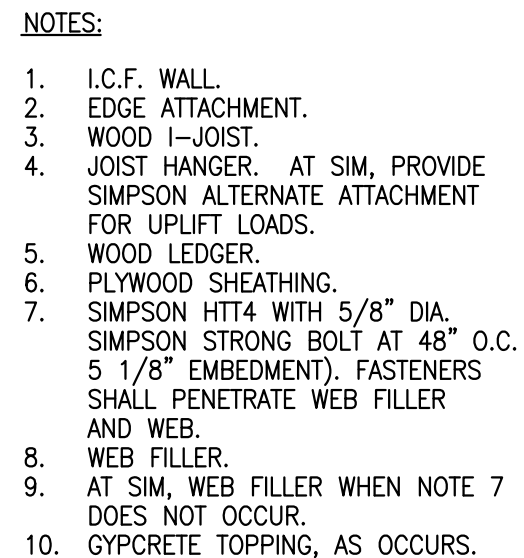
207 WOOD I-JOIST AT WOOD BEAM

215078-S4.0-202



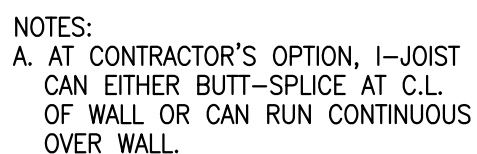
204 WOOD BEAM AT I.C.F. WALL

215078-S4.0-204



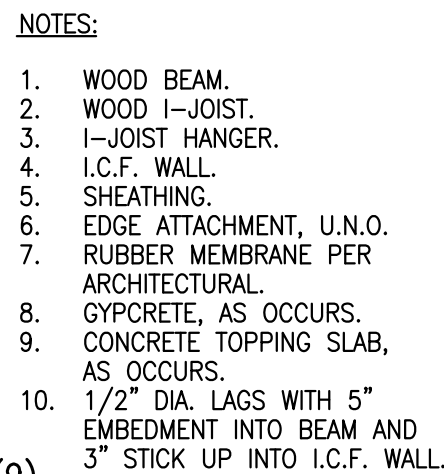
201 WOOD I-JOIST AT I.C.F. WALL

215078-S4.0-201



210 WOOD I-JOIST AT WOOD STUD WALL

215078-S4.0-210



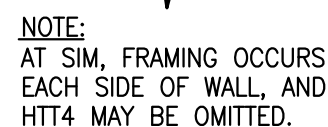
211 I.C.F. WALL AT WOOD BEAM

215078-S4.0-211



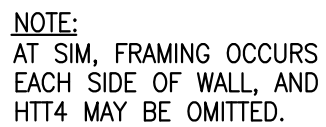
208 WOOD TRUSS AT I.C.F. WALL

215078-S4.0-208



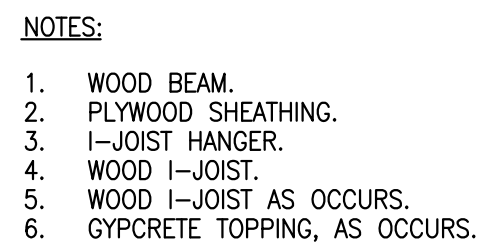
205 WOOD I-JOIST AT I.C.F. WALL

215078-S4.0-205



206 WOOD TRUSS AT I.C.F. WALL

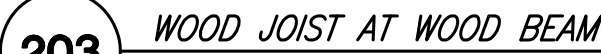
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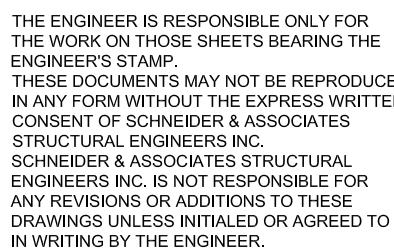
202 WOOD I-JOIST AT WOOD BEAM

215078-S4.0-202

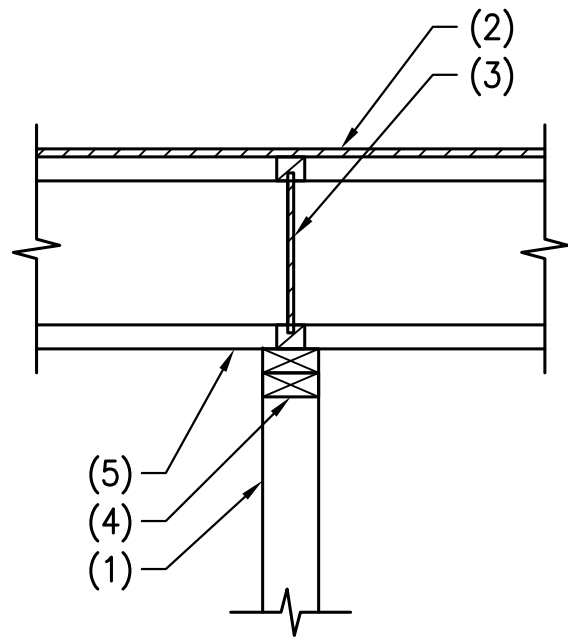
B. 6/24/15



624-030



S4.0
sheet



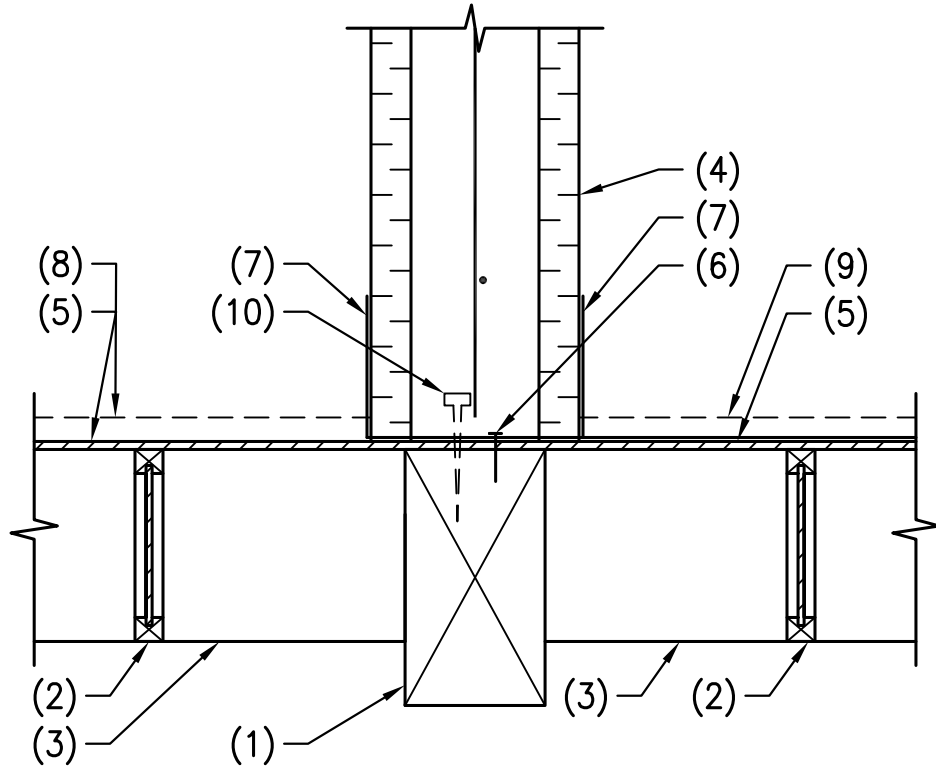
- NOTES:
1. WOOD STUD WALL.
 2. PLYWOOD SHEATHING.
 3. WOOD I-JOIST BLOCKING.
 4. DOUBLE 2x TOP PLATE.
 5. WOOD I-JOIST.

NOTES:
A. AT CONTRACTOR'S OPTION, I-JOIST CAN EITHER BUTT-SPLICE AT C.L. OF WALL OR CAN RUN CONTINUOUS OVER WALL.

215 WOOD I-JOIST AT WOOD STUD WALL

SCALE: NOT TO SCALE

215078-S4.0-215

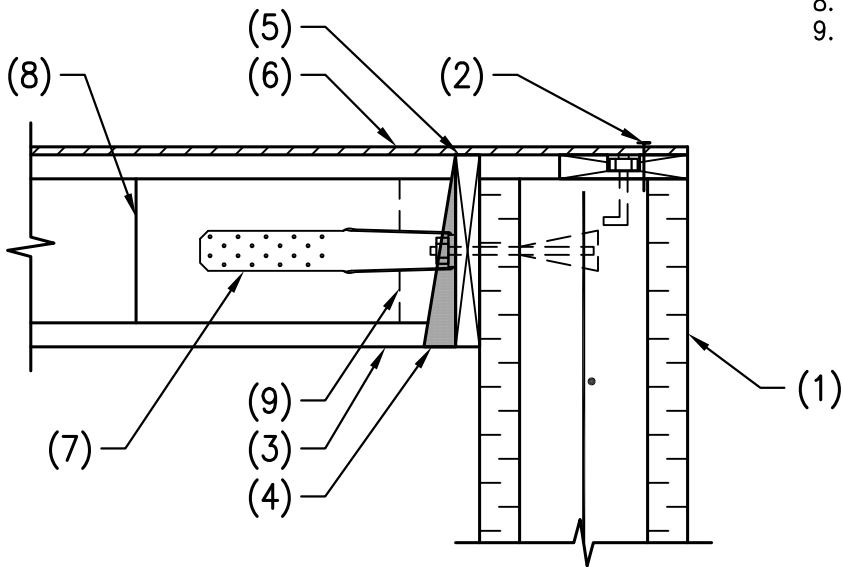


- NOTES:
1. WOOD BEAM.
 2. WOOD I-JOIST.
 3. WOOD LSL RIM BOARD AT 48" O.C. WITH #8x2 1/2" SCREWS AT 6" O.C. STOP AFTER 3 JOIST BAYS.
 4. I.C.F. WALL.
 5. SHEATHING.
 6. EDGE ATTACHMENT, U.N.O.
 7. RUBBER MEMBRANE PER ARCHITECTURAL.
 8. GYPCRETE, AS OCCURS.
 9. CONCRETE TOPPING SLAB, AS OCCURS.
 10. 1/2" DIA. LAGS WITH 5" EMBEDMENT INTO BEAM AND 3" STICK UP INTO I.C.F. WALL.

212 I.C.F. WALL AT WOOD BEAM

SCALE: NOT TO SCALE

215078-S4.0-212



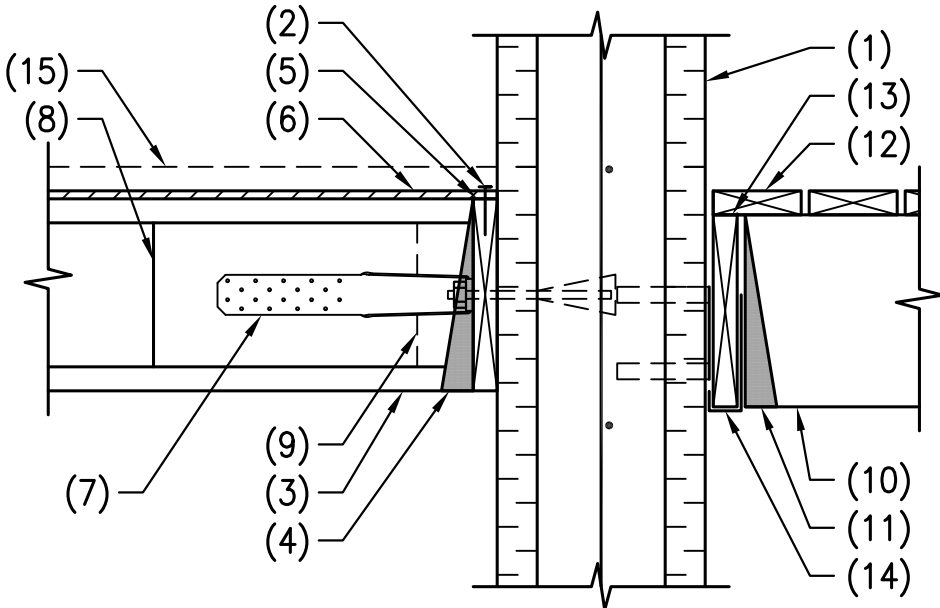
- NOTES:
1. I.C.F. WALL.
 2. EDGE ATTACHMENT.
 3. WOOD I-JOIST.
 4. JOIST HANGER. AT SIM, PROVIDE SIMPSON ALTERNATE ATTACHMENT FOR UPLIFT LOADS.
 5. WOOD LEDGER.
 6. PLYWOOD SHEATHING.
 7. SIMPSON HT4 WITH 5/8" DIA. SIMPSON STRONG BOLT AT 48" O.C. 5 1/8" EMBEDMENT). FASTENERS SHALL PENETRATE WEB FILLER AND WEB.
 8. WEB FILLER.
 9. AT SIM, WEB FILLER WHEN NOTE 7 DOES NOT OCCUR.

213 WOOD I-JOIST AT I.C.F. WALL

SCALE: NOT TO SCALE

215078-S4.0-201

- NOTES:
1. I.C.F. WALL.
 2. EDGE ATTACHMENT.
 3. WOOD I-JOIST.
 4. JOIST HANGER. AT SIM, PROVIDE SIMPSON ALTERNATE ATTACHMENT FOR UPLIFT LOADS.
 5. WOOD LEDGER.
 6. PLYWOOD SHEATHING.
 7. SIMPSON HT4 WITH 5/8" DIA. SIMPSON STRONG BOLT AT 48" O.C. 5 1/8" EMBEDMENT). FASTENERS SHALL PENETRATE WEB FILLER AND WEB.
 8. WEB FILLER.
 9. AT SIM, WEB FILLER WHEN NOTE 7 DOES NOT OCCUR.
 10. 2x DECK JOIST.
 11. JOIST HANGER.
 12. 2x DECKING.
 13. WOOD LEDGER.
 14. LEDGER CONNECTION.
 15. GYPCRETE TOPPING, AS OCCURS.



214 WOOD I-JOIST AT I.C.F. WALL

SCALE: NOT TO SCALE

215078-S4.0-214

REVIEWED FOR STRUCTURAL CODE COMPLIANCE

R. Branch 6/24/15

JOHNSON RESIDENCE
15-3A RESOLUTION POINTE
ANCHORAGE, ALASKA

FRAMING DETAILS



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project 215078
engineer W. OVIATT
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