

3D Johnson Residen

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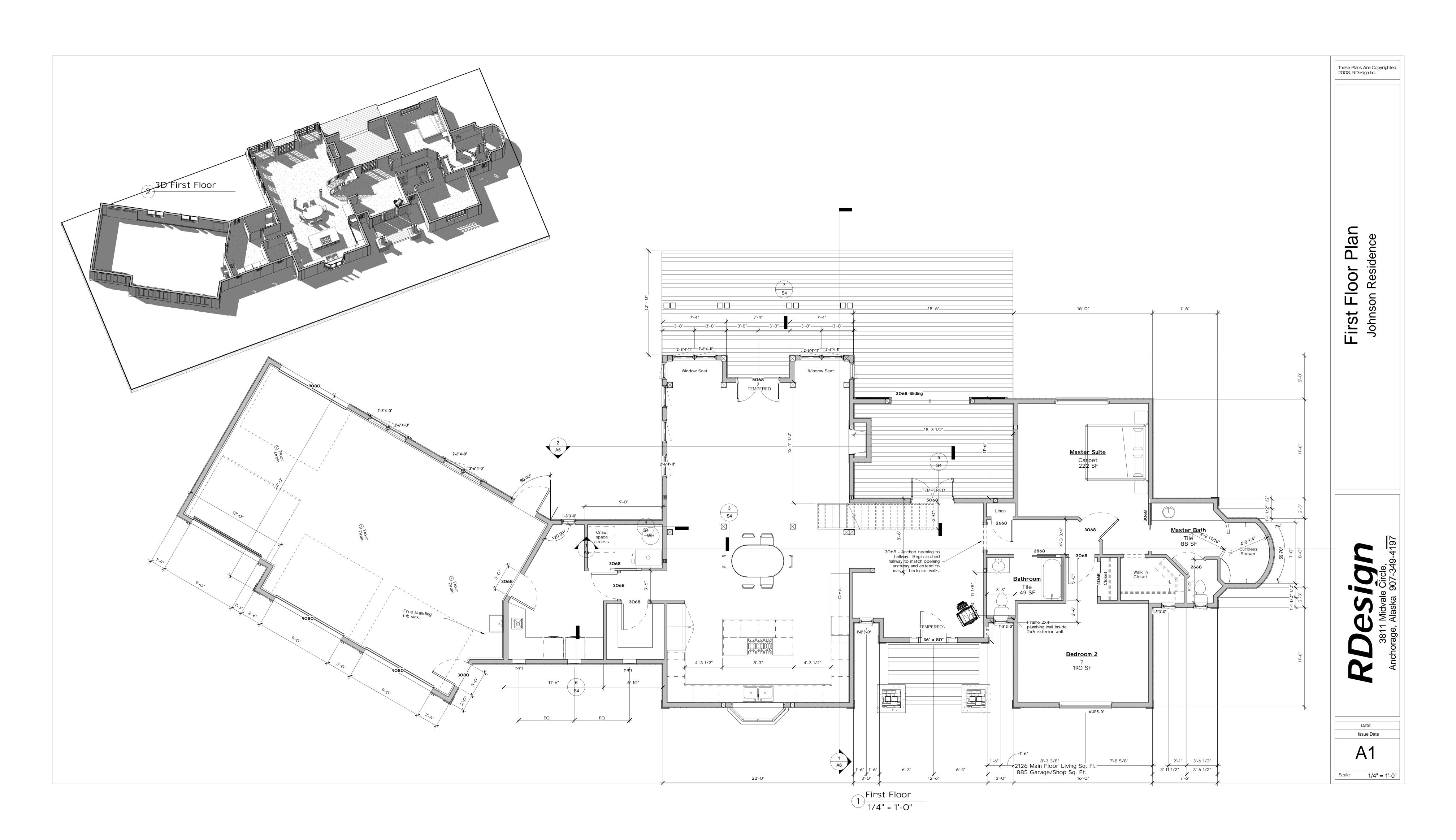
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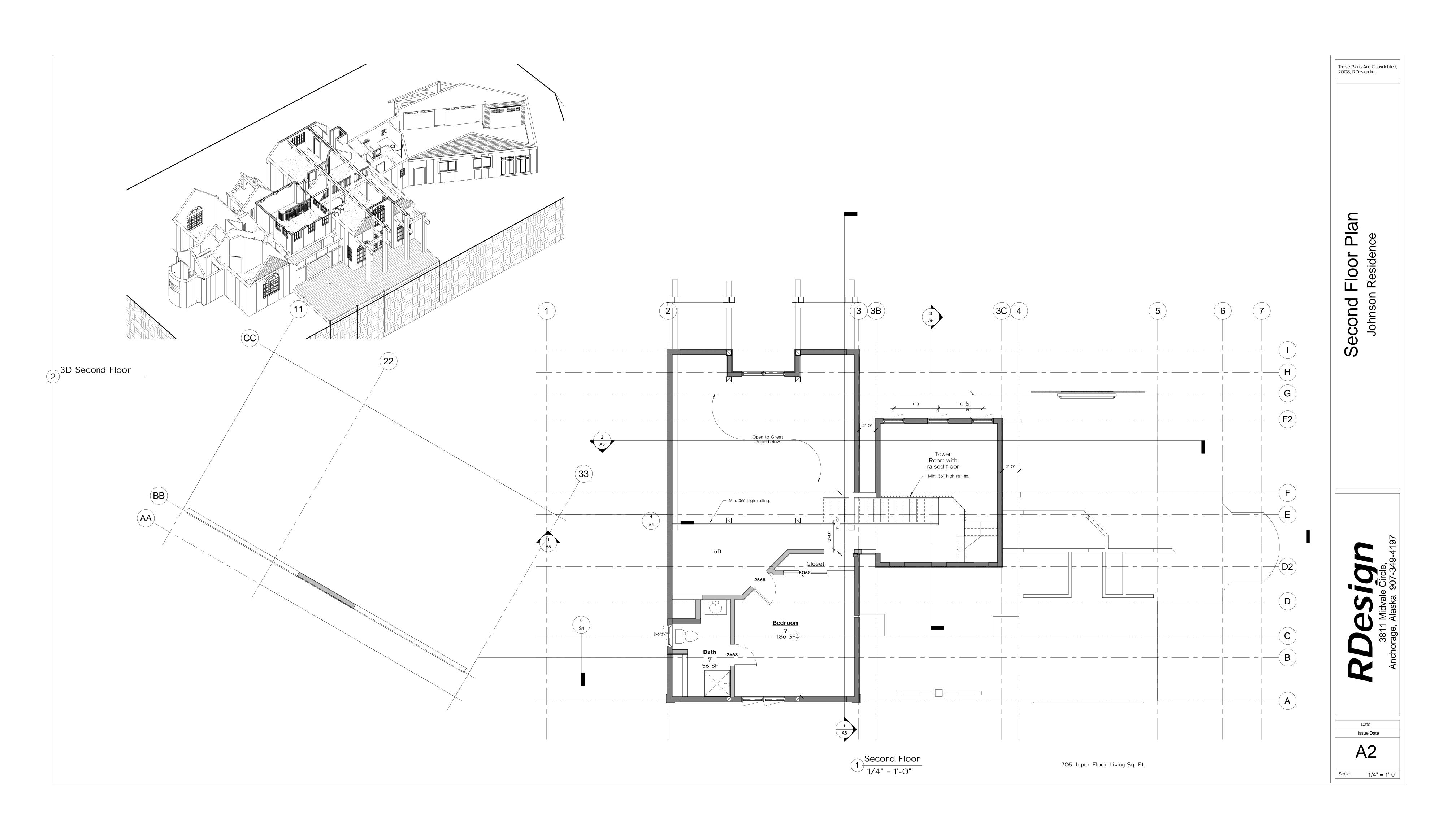
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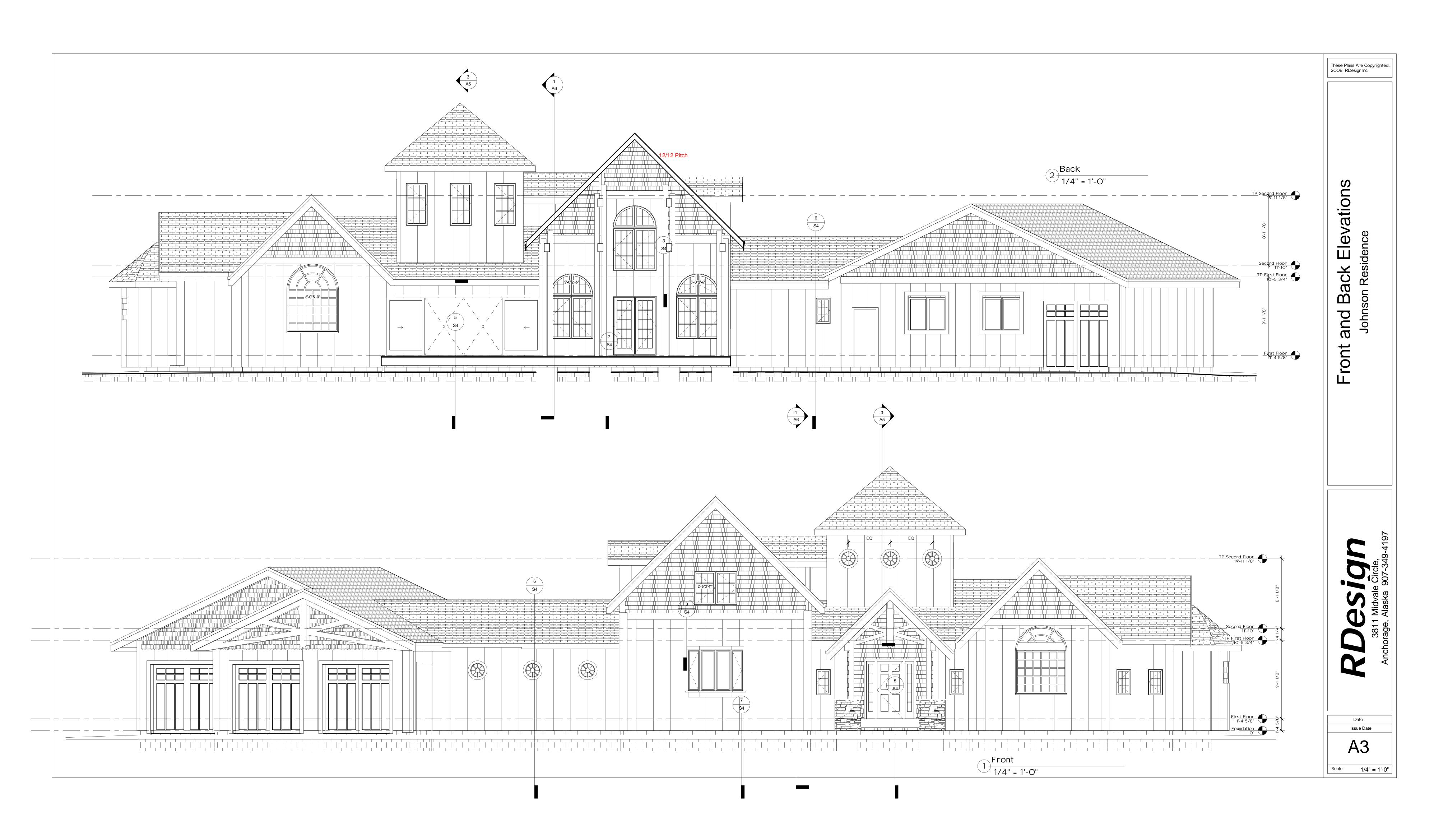
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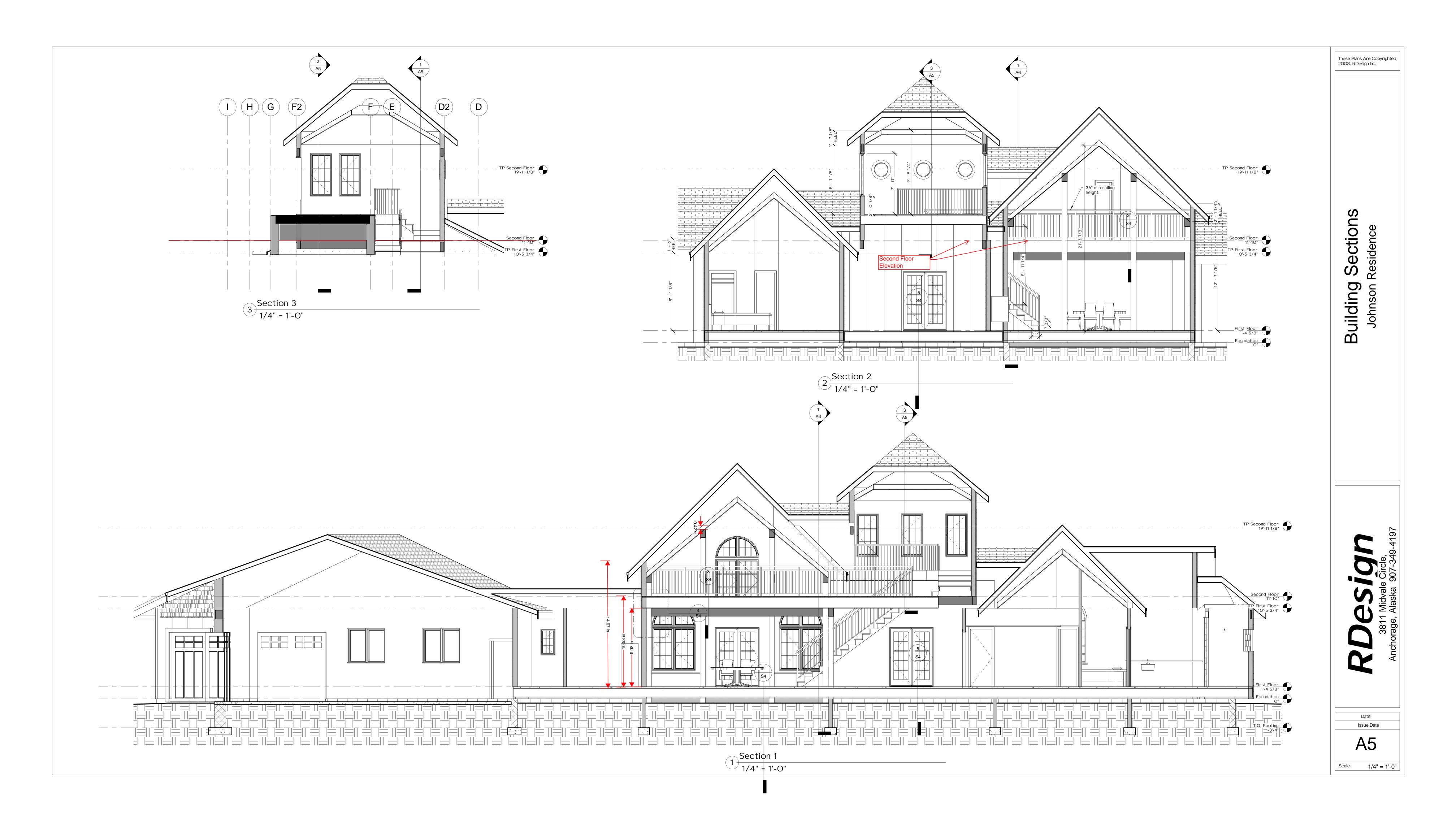
Left and Right Elevations
Johnson Residence

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Date
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Scale 1/4" = 1'-0"



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Building Sections
Johnson Residence

RDESIGN 3811 Midvale Circle, Anchorage, Alaska 907-349-4197

Date Issue Date

A6

Scale 1/4" = 1'-0"

Section 7

GENERAL STRUCTURAL NOTES

(APPLY UNLESS NOTED OTHERWISE)

DESIGN CRITERIA:

2009 EDITION OF THE INTERNATIONAL BUILDING CODE, WITH LOCAL AMENDMENTS.

LOADS:

- 1. BUILDING OCCUPANCY CATEGORY II.
- 2. FLOOR LIVE LOAD = 40 PSF
- 3. SUPERIMPOSED DEAD LOAD ON ROOF TRUSSES = 15 PSF (10 PSF TOP CHORD, 5 PSF BOTTOM CHORD).
- 4. GROUND SNOW LOAD = 50 PSF, Ce=1.0, Is=1.0, Ct=1.0, FLAT ROOF SNOW LOAD = 40 PSF.
- 5. WIND SPEED = 100 MPH (3-SECOND GUST), EXPOSURE D, Iw=1.00, GCpi=+/-0.18, Kzt=1.24. COMPONENT AND CLADDING PRESSURES (BASED ON 10 SF TRIBUTARY AREA):
 - -78.20 PSF (ROOF, WITHIN 6 FEET OF ROOF CORNERS) -52.88 PSF (ROOF, WITHIN 6 FEET OF ROOF EDGES)
 - -30.38 PSF (ROOF, ALL OTHER LOCATIONS)
 - -44.44 PSF (WALLS, WITHIN 6 FEET OF WALL CORNERS)
 - -36.01 PSF (WALLS, ALL OTHER LOCATIONS)
- 6. SEISMIC DESIGN CATEGORY D, SOILS SITE CLASS D, le=1.0, Ss=1.50g, S1=0.55g, SDs=1.00g, SD1=0.55g, SPECIAL REINFORCED CONCRETE SHEAR WALLS, EQUIVALENT LATERAL FORCE PROCEDURE
- 7. ALL LOADS INDICATED ARE WORKING STRESS LOADS U.N.O.

GENERAL:

- 1. THE STRUCTURAL CONSTRUCTION DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OR SEQUENCE OF CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR AND PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES, SEQUENCES FOR PROCEDURE OF CONSTRUCTION, OR THE SAFETY PRECAUTIONS AND THE PROGRAMS INCIDENT THERETO (NOR SHALL OBSERVATION VISITS TO THE SITE INCLUDE INSPECTION OF THESE ITEMS). THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND IMPLEMENTATION OF ALL SCAFFOLDING, BRACING AND SHORING.
- 2. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED CONSTRUCTION. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT.
- 3. WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE THE LATEST EDITION AND/OR ADDENDA.
- 4. ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL WITH APPROPRIATE TRADES, DRAWINGS AND SUBCONTRACTORS PRIOR TO CONSTRUCTION.
- 5. OPTIONS ARE FOR CONTRACTOR'S CONVENIENCE. IF AN OPTION IS CHOSEN, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL NECESSARY CHANGES AND SHALL COORDINATE ALL DETAILS WITH ALL TRADES.
- 6. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT. FOR BIDDING PURPOSES, WHERE ANY MEMBER IS SHOWN BUT NOT CALLED OUT, THE LARGEST SIMILAR MEMBER SHALL BE UTILIZED.
- 7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION. RESOLVE ANY DISCREPANCY WITH THE ARCHITECT. DO NOT USE SCALED DIMENSIONS.
- 8. ALL DETAILS SHALL BE INCORPORATED INTO THE PROJECT AT ALL APPROPRIATE LOCATIONS, WHETHER SPECIFICALLY CUT OR NOT. TYPICAL DETAILS MAY NOT NECESSARILY BE CUT ON PLANS, BUT APPLY UNLESS NOTED OTHERWISE. FOR CLARITY, DETAILS MAY SHOW ONLY ONE SIDE OF FRAMING CONDITION.
- 9. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS, THE GREATER REQUIREMENTS SHALL GOVERN.
- 10. ANY ENGINEERING DESIGN, PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW, SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF ALASKA.

FOUNDATIONS:

- 1. FOUNDATION DESIGN IS BASED ON SOILS REPORT BY NORTHERN GEOTECHNICAL ENGINEERING, INC./TERRA FIRMA TESTING, REPORT NUMBER 1322-06. ALL CONSTRUCTION SHALL CONFORM TO THE RECOMMENDATIONS OF THE SOILS REPORT.
- 2. SPREAD FOOTINGS SHALL BEAR ON NATIVE SOIL AND/OR ENGINEERED FILL PER THE SOILS REPORT. MAXIMUM ALLOWABLE SOIL BEARING PRESSURE = 2,500 PSF, WITH 1/3 INCREASE FOR LOAD COMBINATIONS INCLUDING WIND OR SEISMIC LOADS.
- 3. MINIMUM FOOTING BEARING DEPTHS SHALL BE AS FOLLOWS:
 - UNHEATED EXTERIOR FOOTINGS ----- 60" BELOW FINISHED GRADE*
 HEATED PERIMETER FOOTINGS ----- 42" BELOW FINISHED GRADE
 INTERIOR (CRAWL SPACE) FOOTINGS -----12" BELOW FINISHED GRADE
 - * IF NON-FROST-SUSCEPTIBLE BEARING SOILS CANNOT BE VERIFIED, INCREASE

BEARING DEPTH TO 120" BELOW FINISHED GRADE.

- 4. PROVIDE POSITIVE DRAINAGE SLOPES, BOTH DURING AND AFTER CONSTRUCTION, FOR SURFACE AND ROOF RUNOFF, MINIMUM 10'-0" FROM BUILDING FOUNDATIONS.
- 5. DO NOT BACKFILL AGAINST BASEMENT OR RESTRAINED WALLS UNTIL FRAMING TO SUPPORT WALL IS PERMANENTLY ATTACHED. DO NOT EXCEED 1'-0" DIFFERENTIAL IN FILL LEVEL ON OPPOSITE SIDES OF FOUNDATION WALLS.
- 6. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR ANY GEOTECHNICAL ASPECTS OF THIS PROJECT. THE OWNER SHALL EMPLOY A REGISTERED GEOTECHNICAL ENGINEER TO PERFORM NECESSARY TESTING AND QUALITY CONTROL INSPECTIONS TO ENSURE THAT THE RECOMMENDATIONS OF THE SOILS REPORT ARE MET. ALL EARTHWORK SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER.
- 7. SEE ARCHITECTURAL/CIVIL DRAWINGS FOR EXTERIOR SLABS AND SIDEWALKS.

CONCRETE:

1. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 301 AND ACI 318. CEMENT SHALL CONFORM TO ASTM C150, TYPE II. AGGREGATE SHALL CONFORM TO ASTM C33. CONCRETE SHALL BE READY MIXED IN ACCORDANCE WITH ASTM C94 AND SHALL BE DESIGNED FOR A MINIMUM 28-DAY COMPRESSIVE STRENGTH AS FOLLOWS:

SLABS ON GRADE	3,000 P	SI
FOUNDATIONS	3,000 P	
WALLS	3,000	PS

- 2. CONCRETE SHALL BE FREE OF CHLORIDE. FOR I.C.F. WALLS, CONCRETE SLUMP AND W/C RATIO SHALL MEET THE I.C.F. MANUFACTURER'S SPECIFICATIONS. PLASTICIZER MAY BE USED AS REQUIRED TO ACHIEVE ADEQUATE SLUMP. DO NOT ADD WATER ON SITE TO INCREASE SLUMP OR WORKABILITY.
- 3. MECHANICALLY CONSOLIDATE ALL CONCRETE WHEN PLACED, EXCEPT SLABS ON GRADE DO NOT DROP CONCRETE MORE THAN FIVE FEET WITH OUT THE USE OF TREMIES.
- 4. PROVIDE SLEEVES FOR ALL UTILITY OPENINGS. DO NOT CUT ANY REINFORCING AT OPENINGS. CONCRETE WHICH HAS CONTAINED WATER FOR MORE THAN 90 MINUTES SHALL NOT BE USED. RETEMPERING OF CONCRETE AFTER INITIAL SET IS NOT ALLOWED. CURE EXPOSED CONCRETE PER ACI 301 FOR A MINUMUM OF 7 DAYS.

INSULATED CONCRETE FORMS (I.C.F.):

- 1. CONCRETE WALLS SHALL BE FORMED WITH I.C.F. THE I.C.F. SYSTEM SHALL HAVE CURRENT ICC APPROVAL. COORDINATE INSULATION REQUIREMENTS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- 2. THE I.C.F. SYSTEM SHALL PROVIDE A CONTINUOUS FLAT CONCRETE WALL PANEL OF THE THICKNESS(ES) SPECIFIED IN THE DRAWINGS.
- 3. PLACE REINFORCING AS SPECIFIED IN THE DRAWINGS. COORDINATE REINFORCING LOCATION WITH THE PRE-SET LOCKS PROVIDED IN THE I.C.F. WEBS. CENTER VERTICAL REINFORCING IN WALLS U.N.O.
- 4. I.C.F. LEDGER CONNECTORS SHALL BE THE ICFVL LEDGER CONNECTOR SYSTEM BY SIMPSON STRONG—TIE INC. INSTALL PER THE MANUFACTURER'S INSTRUCTIONS. FOR OTHER ANCHOR CONNECTIONS, NOTCH THE INSULATION AROUND THE ANCHOR BODY, TO PROVIDE 3 INCHES OF CLEAR CONCRETE COVER AROUND THE ANCHOR BODY U.N.O.
- 5. CLEAN ALL CAVITIES OF DEBRIS PRIOR TO PLACING CONCRETE. PROVIDE CLEAN—OUTS AS REQUIRED. REPAIR CLEAN—OUTS WITH AN EQUIVALENT AMOUNT OF INSULATION.

REINFORCING STEEL:

- 1. REINFORCING STEEL SHALL CONFORM TO ASTM A615 (Fy = 60 KSI) DEFORMED BARS FOR ALL BARS #4 AND LARGER. ASTM A615 (Fy = 40 KSI) DEFORMED BARS FOR ALL BARS #3 AND SMALLER.
- 2. ACCURATELY PLACE OR SUPPORT ALL REINFORCING, WITH GALVANIZED METAL CHAIRS, SPACERS OR HANGERS FOR THE FOLLOWING CLEAR CONCRETE COVERAGES:

CAST AGAINST AND PERMANE	 RTH 3"
#6 OR LARGER	 -
#5 AND SMALLER ALL OTHER PER LATEST EDI	1 1/2"

- 3. SEE DRAWINGS FOR SIZE AND SPACING OF REINFORCING. LAP SPLICE ALL REINFORCING 60 BAR DIAMETERS MINIMUM.
- 4. ALL SPLICE LOCATIONS ARE SUBJECT TO APPROVAL BY THE STRUCTURAL ENGINEER. SPLICED BARS SHALL BE PLACED AT THE SAME EFFECTIVE DEPTH U.N.O. ALL REINFORCING NOTED AS "CONTINUOUS" SHALL BE FULLY CONTINUOUS AND SPLICED PROVIDE BENT CORNER BARS TO MATCH AND LAP WITH HORIZONTAL BARS AT ALL CORNERS AND INTERSECTIONS PER TYPICAL DETAILS.
- 5. REINFORCING BAR SPACING GIVEN ARE MAXIMUM ON CENTERS. ALL BARS PER CRSI SPECIFICATIONS AND HANDBOOK. DOWEL ALL VERTICAL REINFORCING TO FOUNDATION WITH STANDARD 90 DEGREE HOOKS UNLESS NOTED OTHERWISE. SKEW HOOKS AS REQUIRED TO MAINTAIN CONCRETE COVER. SECURELY TIE ALL BARS IN LOCATION BEFORE PLACING CONCRETE. AT I.C.F. CONSTRUCTION, TIES MAY BE OMITTED ONLY IF THE REINFORCING CAN BE LOCKED INTO PLACE USING THE I.C.F.'S PRE-SET REINFORCING LOCKS.
- 6. CONTRACTOR'S OPTION TO REINFORCE SLABS ON GRADE WITH FIBERMESH REINFORCING FIBERS MANUFACTURED BY FIBERMESH COMPANY, OR EQUIVALENT. FIBERS SHALL BE 100% VIRGIN POLYPROPYLENE, FIBRILLATED FIBERS CONTAINING NO REPROCESSED OLEFIN MATERIALS AND SPECIFICALLY MANUFACTURED TO AN OPTIMUM GRADATION UTILIZING 25 INDIVIDUAL FIBER DESIGNS FOR USE AS CONCRETE SECONDARY REINFORCEMENT. MIX AT A RATE OF 1.5 POUNDS PER CUBIC YARD.

INTERPRETATION OF DRAWINGS

	PLAN LEGEND						
SYMBOL	DESCRIPTION	REMARKS					
101	DETAIL CUTS SHOWN ON PLANS	FOUNDATION DETAILS ARE 100 SERIES NUMBERS. FRAMING DETAILS ARE 200 SERIES NUMBERS					
1	KEYNOTE						
	I.C.F. WALL U.N.O.	SEE PLANS AND SCHEDULES FOR SIZE AND REINFORCING					
	WOOD STUD WALL U.N.O.	SEE PLANS AND SCHEDULES FOR SIZE					
H ▼	HOLDOWN ANCHOR	SEE PLANS FOR LOCATION AND TYPE					
7////	SLAB DEPRESSION/CHANGE IN ELEVATION	VERIFY DEPTH WITH ARCHITECTURAL DRAWINGS					
<u>SLOPE</u>	DIRECTION OF SLOPE	VERIFY SLOPE WITH ARCHITECTURAL AND/OR MECHANICAL DRAWINGS					
	REVISION SYMBOL						
	OPENING						

	ABBREV	'IATIONS	
A.B.C. —	– AGGREGATE BASE COURSE	LBS —	– POUNDS
A.F.F. ———	— ABOVE FINISHED FLOOR	L.L.H. ———	 LONG LEG HORIZONTAL
ALT ———	— ALTERNATE	L.L.V. ———	 LONG LEG VERTICAL
A.B. ———	- ANCHOR BOLT	MFR('S)	MANUFACTURER('S)
B.F.F	— BELOW FINISHED FLOOR	М.С.Ĵ. ´ ——	— MASONRY CONTROL JOINT
B.O.B. ——	- BOTTOM OF BEAM	MECH. ——	- MECHANICAL
B.O.D. ——	- BOTTOM OF DECK	N/A	- NOT APPLICABLE
B.O.F. ——	- BOTTOM OF FOOTING	N.F.S. ———	- NON-FROST SUSCEPTIBLE
B.O.S. ——	- BOTTOM OF STEEL	N.T.S. ———	- NOT TO SCALE
BOT. ———	- BOTTOM	0.C. —	ON CENTER
BRG ——	- BEARING	OPP ———	- OPPOSITE (MIRRORED)
C.I.P. ———	- CAST IN PLACE	P.A.F. ———	- POWDER ACTUATED FASTENER
C.L. —	- CENTERLINE	P.C. —	- PRECAST CONCRETE
		PCF ———	- POUNDS PER CUBIC FOOT
CLR —	- CLEAR	PLF —	- POUNDS PER CUBIC FOOT - POUNDS PER LINEAR FOOT
CONC.	- CONCRETE	_	
	- CONCRETE CONTROL JOINT	PREFAB. ——	- PREFABRICATED
	- CONCRETE MASONRY UNIT	PSF —	- POUNDS PER SQUARE FOOT
CONN. —	- CONNECTION	PSI ———	- POUNDS PER SQUARE INCH
CONT. —	- CONTINUOUS	REINF.	- REINFORCING
D.L. ———	— DEAD LOAD	SCH —	- SCHEDULE
DIA. ———	— DIAMETER	SIM —	— SIMILAR
DN	— DOWN		— STRUCTURAL INSULATED PANEI
DWG(S) —	- DRAWING(S)	SP —	— SPACES
(E) ———	- EXISTING	STD ——	— STANDARD
È.F. ———	— EACH FACE	Т & В ——	— TOP AND BOTTOM
E.O.S. ———	— EDGE OF SLAB	T.L. ———	— TOTAL LOAD
EQ	– EQUAL	T.O.B. ———	- TOP OF BEAM
EQUIP. ——		T.O.C. ———	— TOP OF CONCRETE
	- EXPANSION BOLT		— TOP OF DECK
	- EXPANSION JOINT		- TOP OF FOOTING
E.W. ———			- TOP OF LEDGER
	- FINISHED FLOOR ELEVATION		- TOP OF MASONRY
GA ———			- TOP OF PLATE
GALV. ———			- TOP OF STEEL
	— GALVANIZED — GENERAL STRUCTURAL NOTES		- TOP OF WALL
	GLUED-LAMINATED BEAM	TYP ———	
			· · · · - · —
HORIZ.	- HORIZONTAL - INTERNATIONAL BUILDING CODE		- UNLESS NOTED OTHERWISE
		VERT.	
	- INTERNATIONAL CODE COUNCIL		- WOOD STRUCTURAL PANEL
	- INSULATED CONCRETE FORM		- WELDED WIRE FABRIC
· /	- 1000 POUNDS	W/	
L.L. ———	- LIVE LOAD	w/o ——	- WITHOUT

REVIEWED FOR STRUCTURAL CODE COMPLIANCE



SCHNEIDER STRUCTURAL ENGINEERS
FIRM DEDICATION TO STRUCTURAL INNOVATION
8811 Toloff Street | Anchorage, AK 99507 | 907.561.2135 | www.sastructural.com

JOHNSON RESIDENCE
15-3A RESOLUTION POINTE
ANCHORAGE, ALASKA
GENERAL STRUCTURAL NOTES AND
INTERPRETATION OF DRAWINGS



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project 215078

number date

 engineer
 W. OVIATT

 drafter
 R. BRANCH

 date
 06/17/15

S1.0

GENERAL STRUCTURAL NOTES (CONTINUED)

(APPLY UNLESS NOTED OTHERWISE)

POST-INSTALLED ANCHORS:

- 1. INSTALL POST-INSTALLED ANCHORS ONLY AS INDICATED ON THE DRAWINGS AND/OR WITH SPECIFIC WRITTEN APPROVAL OF THE ENGINEER PRIOR TO INSTALLATION.
- 2. EPOXY ANCHORS IN CONCRETE SHALL BE THREADED ROD OR REINFORCING STEEL INSTALLED WITH HIT-HY 200-R ADHESIVE BY HILTI PER I.C.C. REPORT ESR-3187.
- 3. EXPANSION ANCHORS IN CONCRETE SHALL BE KWIK BOLT TZ BY HILTI, INSTALLED PER I.C.C. REPORT ESR-1917.
- 4. SCREW ANCHORS IN CONCRETE SHALL BE TITEN HD BY SIMPSON, INSTALLED PER ICC REPORT ESR-2713.
- 5. THE CONTRACTOR MAY NOT USE SUBSTITUTES FOR POST-INSTALLED ANCHORS WITHOUT PRIOR APPROVAL OF THE ENGINEER.
- 6. SEE DRAWINGS FOR ANCHOR EMBEDMENT DEPTHS. INSTALL ANCHORS AS OUTLINED IN THE MANUFACTURER'S SPECIFICATIONS, UTILIZING PROPER SIZE AND TYPE OF DRILL AND PROPER HOLE CLEANING, DRIVING, AND TIGHTENING TECHNIQUES.

ROUGH CARPENTRY AND PLYWOOD:

1. ALL FRAMING PER IBC CHAPTER 23. FRAMING LUMBER SHALL COMPLY WITH THE 2005 EDITION OF THE NATIONAL DESIGN SPECIFICATION. MAXIMUM MOISTURE CONTENT SHALL NOT EXCEED 19 PERCENT. ALL SAWN LUMBER SHALL BE STAMPED WITH THE GRADE MARK OF AN APPROVED LUMBER GRADING AGENCY. ALL SAWN LUMBER SHALL BE OF THE FOLLOWING MINIMUM GRADES:

MEMBER TYPE	WOOD TYPE
JOISTS 2x4 2x6 OR LARGER	D.F. STUD D.F. #2
BEAMS, LEDGERS, LINTELS AND TOP PLATES WIDTH 4" OR LESS WIDTH GREATER THAN 4"	D.F. #2 D.F. #1
STUDS, PLATES AND BLOCKING 2x4 2x6 OR LARGER	D.F. STND D.F. #2
POSTS 4x4 4x6 OR LARGER 6x6 OR LARGER	D.F. #2 D.F. #2 D.F. #1

2. ALL PLYWOOD SHALL CONFORM TO PS-1 OR APA PRP-108, SHALL HAVE AN EXTERIOR OR EXPOSURE 1 CLASSIFICATION AND SHALL BEAR THE STAMP OF AN APPROVED TESTING AGENCY. LAY UP PLYWOOD WITH FACE GRAIN PERPENDICULAR TO SUPPORTS AND STAGGER JOINTS. ON ROOFS WHERE PLYWOOD IS LAID UP WITH FACE GRAIN PARALLEL TO SUPPORTS, USE A MINIMUM OF 5-PLY PLYWOOD. ALL PLYWOOD SHALL BE OF THE FOLLOWING NOMINAL THICKNESS, SPAN/INDEX RATIO AND SHALL BE ATTACHED AS FOLLOWS UNLESS NOTED OTHERWISE:

USE	THICKNESS	SPAN/INDEX RATIO	EDGE ATTACHMENT	INTERMEDIATE ATTACHMENT
ROOF	5/8"	32/16	10d AT 6" O.C	- 10d AT 12" O.C.
FLOOR* - 3/4	-" T & G	48/24	10d AT 6" O.C	- 10d AT 10" O.C.

- * AT FLOORS, USE RING SHANK NAILS. IN ADDITION TO NAILING, GLUE ALL FLOOR SHEATHING TO FRAMING WITH AN APA AFG-01 QUALIFIED GLUE.
- 3. AMERICAN PLYWOOD ASSOCIATION PERFORMANCE RATED SHEATHING (OSB) MAY BE USED AS AN ALTERNATE TO PLYWOOD WITH PRIOR WRITTEN APPROVAL OF ARCHITECT. WHERE ROOF IS TO BE GUARANTEED, IT MAY NOT BE USED WITHOUT PRIOR APPROVAL FROM BUILT-UP ROOF SYSTEM MANUFACTURER. RATED SHEATHING SHALL COMPLY WITH I.C.C. REPORT NER-108, HAVE AN EXTERIOR OR EXPOSURE 1 CLASSIFICATION, AND SHALL HAVE A SPAN RATING EQUIVALENT TO OR BETTER THAN THE PLYWOOD IT REPLACES. ATTACHMENT AND THICKNESS (WITHIN 1/32") SHALL BE THE SAME AS THE PLYWOOD IT REPLACES. INSTALL PER MANUFACTURER'S RECOMMENDATIONS.
- 4. SEE TYPICAL DETAILS FOR ALLOWABLE NOTCHES AND HOLES IN STRUCTURAL FRAMING. DO NOT NOTCH, DRILL, OR SPLICE JOISTS, BEAMS, OR LOAD BEARING STUDS BEYOND THE DETAILS WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.
- 5. ALL LUMBER IN CONTACT WITH CONCRETE OR MASONRY FOUNDATIONS SHALL BE PRESSURE TREATED WOOD STAMPED BY AN APPROVED AGENCY. ALL UNTREATED LUMBER SHALL BE ISOLATED FROM CONCRETE OR MASONRY WITH 2 LAYERS OF 15# ROOFING FELT WITH OVERLAPPING JOINTS.
- 6. METAL FRAMING CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC. OR OTHER MANUFACTURER WITH CURRENT AND EQUIVALENT I.C.C. APPROVAL. ALL NAIL HOLES IN CONNECTORS SHALL BE FILLED WITH NAIL OF THE LARGEST SIZE INDICATED IN THE MANUFACTURER'S CATALOG U.N.O.

7. ALL NAILING NOT NOTED SHALL BE ACCORDING TO TABLE 2304.9.1 OF THE IBC. NAILS SHALL BE COMMON NAILS WITH THE FOLLOWING TYPICAL DIMENSIONS (PER ASTM F1667) UNLESS NOTED OTHERWISE:

(L = LENGTH, D = SHANK DIAMETER, H = HEAD DIAMETER)

6d	L 2" D 0.113" H 0.266"	16d	L 3-1/2" D 0.162" H 0.344"
8d	L 2-1/2" D 0.131" H 0.281"	20d	L 4" D 0.192" H 0.406"
10d	L 3" D 0.148" H 0.312"	30d	L 4-1/2" D 0.207" H 0.432"

- 8. WHERE SCREWS ARE INDICATED FOR WOOD TO WOOD ATTACHMENTS, USE WOOD SCREWS WITH TYPICAL DIMENSIONS PER ANSI B18.6.1
- 9. ALL FASTENERS IN CONTACT WITH TREATED LUMBER SHALL BE HOT-DIP GALVANIZED OR STAINLESS STEEL.

WOOD STAIRS:

- 1. WOOD STAIRS SHALL BE ASSEMBLED AND FABRICATED BY QUALIFIED CARPENTERS. ALL FRAMING IS PER IBC CHAPTER 23. MAXIMUM MOISTURE CONTENT SHALL NOT EXCEED 16%.
- 2. WOOD STAIR STRINGERS SHALL BE OF THE FOLLOWING MINIMUM SIZE AND SPECIES:
 - A. SOLID SAWN WOOD STRINGERS: 2x12 H.F. #2
 - B. ENGINEERED WOOD STRINGERS: 1-3/4" WIDE x 11-1/4" DEEP WITH A MINIMUM Fb = 2,600 PSI AND MINIMUM MODULUS OF ELASTICITY OF 1,700,000 PSI.
- 3. WOOD STAIR STRINGERS AS DETAILED ABOVE MAY BE USED FOR THE FOLLOWING SPANS AS MEASURED ALONG THE SLOPE OF THE STRINGER:
 - A. SOLID SAWN WOOD STRINGERS: (1) AT 12" O.C. MAXIMUM SPAN = 8'-6". (2) AT 12" O.C. - MAXIMUM SPAN = 12'-6".
 - B. ENGINEERED WOOD STRINGERS: (1) AT 12" O.C. MAXIMUM SPAN = 11'-6" (2) AT 12"O.C. - MAXIMUM SPAN = 14'-6".
 - IF THE PROJECT CONDITIONS EXCEED THOSE NOTED ABOVE, AN ENGINEERED DESIGN COMPLYING WITH THE IBC SHALL BE SEALED BY A REGISTERED ENGINEER AND SUBMITTED FOR REVIEW PRIOR TO ASSEMBLY.
- MAXIMUM NOTCH (AS MEASURED PERPENDICULAR TO THE STRINGER PLANE) SHALL NOT EXCEED 5-3/4" FOR THE STRINGER SPANS SHOWN ABOVE.
- STAIR TREADS SHALL COMPLY WITH THE FOLLOWING MINIMUM PROPERTIES: 3/4" PLYWOOD (OR OSB), 48/24 SPAN RATING OR 1x H.F. #2 FLAT WOOD DECKING.
- 6. ATTACHMENT TO ADJACENT WOOD OR MASONRY WALLS SHALL BE PER DETAILS. COORDINATE ATTACHMENT WITH ARCHITECTURAL FOR SOUND TRANSMISSION PRIOR TO CONNECTING STAIR STRINGERS TO ADJACENT WALLS.
- 7. FOR STAIR DIMENSIONS (INCLUDING RISE AND RUN) AND TREAD SURFACES, SEE ARCHITECTURAL DRAWINGS.

GLUED-LAMINATED BEAMS (GLU-LAM):

- 1. GLUED-LAMINATED BEAMS SHALL BE DOUGLAS FIR LARCH WITH THE FOLLOWING MINIMUM PROPERTIES: Fb = 2,400 PSI, Fv = 190 PSI, Fc (PERPENDICULAR) = 650 PSI, E = 190 PSI1.800.000 PSI. BEAMS CANTILEVERING OVER SUPPORTS SHALL HAVE THE SPECIFIED MINIMUM PROPERTIES TOP AND BOTTOM.
- 2. CAMBER AS SHOWN ON DRAWINGS. IF NO CAMBER IS SPECIFIED, PROVIDE STANDARD CAMBER USING A RADIUS OF 3,500 FEET.
- 3. ALL BEAMS SHALL BE FABRICATED USING WATERPROOF GLUE. FABRICATION AND HANDLING SHALL BE PER LATEST AITC AND WCLA STANDARDS. BEAMS SHALL BEAR GRADE STAMP AND AITC STAMP AND CERTIFICATE. APPEARANCE GRADE BEAMS SHALL BE USED IF INDICATED ON ARCHITECTURAL DRAWINGS.

ENGINEERED LUMBER PRODUCTS:

- 1. LAMINATED VENEER LUMBER (LVL) PRODUCTS SHALL CONFORM TO APA/EWS PRL-501 AND I.C.C. ESR-1387. LVL MEMBERS SHALL BE "VERSA-LAM" 2.0E AS MANUFACTURED BY BOISE ENGINEERED WOOD PRODUCTS, OR APPROVED EQUIVALENT BY OTHER MANUFACTURER WITH CURRENT AND EQUIVALENT ICC APPROVAL.
- 2. LAMINATED STRAND LUMBER (LSL) PRODUCTS SHALL CONFORM TO ICC ESR-1387. LSL MEMBERS SHALL BE 1-1/4" "VERSA-STRAND" 0.8E OR "VERSA-STUD" 1.7E 2400 AS MANUFACTURED BY BOISE ENGINEERED WOOD PRODUCTS, OR APPROVED EQUIVALENT BY OTHER MANUFACTURER WITH CURRENT AND EQUIVALENT ICC APPROVAL
- 3. I-SERIES FLOOR AND ROOF JOISTS SHALL BE BCI JOISTS MANUFACTURED BY BOISE, OR APPROVED EQUIVALENT BY ANOTHER MANUFACTURER, AND SHALL CARRY ICC APPROVAL FOR THE COMPOSITE SECTION. BRIDGING, BLOCKING, AND WEB STIFFENERS SHALL BE INSTALLED PER THE MANUFACTURER'S INSTRUCTIONS. NAILING NOT OTHERWISE SPECIFIED SHALL BE PER THE MANUFACTURER'S INSTRUCTIONS.

PREFABRICATED WOOD TRUSSES:

- 1. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE COMPLETE DESIGN FABRICATION AND ERECTION PROCEDURES OF ALL TRUSSES, BRIDGING AND/OR BLOCKING PANELS, HANGERS, BRACING, ETC. FOR A COMPLETE INSTALLATION OF THE TRUSS SYSTEM. TRUSS CONFIGURATIONS ARE INDICATED ON THE DRAWINGS. ALL BRACING AND BRIDGING SIZES AND SPACINGS BY TRUSS MANUFACTURER IN ACCORDANCE WITH THE LATEST RECOMMENDATIONS OF THE TRUSS PLATE INSTITUTE.
- TRUSSES SHALL BE DESIGNED AND FABRICATED IN ACCORDANCE WITH IBC CHAPTER 23 TO SUPPORT SELF WEIGHT PLUS LIVE LOAD, SUPERIMPOSED DEAD LOADS, AND LATERAL LOADS STATED IN THE GENERAL STRUCTURAL NOTES OR LOCATED ON PLANS. ROOF TRUSSES SHALL BE DESIGNED TO ACCOMMODATE A FUTURE TOP CHORD DEAD LOAD OF 300 POUNDS AT ANY LOCATION. THE UNIFORM LOADS DO NOT INCLUDE SPECIAL OR ADDITIONAL LOADS NOTED ON THE PLANS OR DETAILS. THE ROOF LOAD DURATION FACTOR IS 1.15.
- 3. LIMIT TOTAL LOAD DEFLECTIONS TO SPAN/240 AT SIMPLE SPANS U.N.O. LIMIT LIVE LOAD DEFLECTIONS TO SPAN/360 AT SIMPLE SPANS U.N.O. ALL TRUSSES SHALL BE CAMBERED FOR 1.5 TIMES THE DESIGN DEAD LOAD
- 4. ADDITIONAL TRUSSES SHALL BE SUPPLIED AS REQUIRED TO SUPPORT MECHANICAL EQUIPMENT.
- 5. ALL CONNECTORS SHALL HAVE CURRENT I.C.C. APPROVAL. ALL TRUSS TO TRUSS CONNECTORS SHALL BE DESIGNED BY THE TRUSS MANUFACTURER. MULTIPLE TRUSS MEMBERS SHALL BE FASTENED TOGETHER TO ALLOW TRANSFER OF SHEAR AND TENSION FORCES (MINIMUM 200 PLF) AT PLYWOOD SHEATHING JOINTS AND TO PREVENT CROSS GRAIN BENDING OF TOP CHORDS. ATTACHMENT SHALL BE A CONTINUOUS 20 GAGE METAL PLATE OR OTHER APPROVED MEANS. METHOD OF ATTACHMENT SHALL BE INDICATED ON SHOP DRAWINGS FOR REVIEW.
- 6. WHERE PERMANENT BRACING OF TRUSS MEMBERS IS REQUIRED BY THE TRUSS DESIGN, IT SHALL BE ACCOMPLISHED BY THE FOLLOWING METHOD: THE TRUSSES SHALL BE DESIGNED SO THAT THE BUCKLING OF ANY INDIVIDUAL TRUSS MEMBER CAN BE RESISTED INTERNALLY BY THE STRUCTURE (E.G. BUCKLING MEMBER T-BRACING, L-BRACING, OF THE INDIVIDUAL TRUSS. THE TRUSS INDIVIDUAL MEMBER BUCKLING REINFORCEMENT SHALL BE INSTALLED AS SHOWN ON THE TRUSS DESIGN DRAWING OR ON SUPPLEMENTAL TRUSS MEMBER BUCKLING REINFORCEMENT DIAGRAMS PROVIDED BY THE TRUSS DESIGNER.
- 7. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS, ERECTION DRAWINGS AND DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER FOR REVIEW PRIOR TO MANUFACTURE. CALCULATIONS AND SHOP DRAWINGS SHALL SHOW ANY SPECIAL DETAILS REQUIRED AT BEARING POINTS.
- 8. ALL FABRICATION SHALL BE PERFORMED ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION.

DEFERRED SUBMITTALS: (PER 2006 IBC 106.3.4.2)

- 1. FOR THE PURPOSES OF THIS SECTION, DEFERRED SUBMITTALS ARE DEFINED AS THOSE PORTIONS OF THE DESIGN THAT ARE NOT SUBMITTED AT THE TIME OF THE APPLICATION AND WHICH ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL WITHIN A SPECIFIED
- 2. DEFERRAL OF ANY SUBMITTAL ITEMS SHALL HAVE PRIOR APPROVAL OF THE BUILDING OFFICIAL. THE ARCHITECT OR ENGINEER OF RECORD SHALL LIST THE DEFERRED SUBMITTALS ON THE PLANS AND THE CONTRACTOR SHALL SUBMIT THE DEFERRED SUBMITTAL DOCUMENTS FOR REVIEW BY THE BUILDING OFFICIAL.
- 3. SUBMITTAL DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ARCHITECT OR ENGINEER OF RECORD A MINIMUM OF 30 DAYS PRIOR TO FABRICATION. THE DOCUMENTS SHALL BE REVIEWED FOR GENERAL CONFORMANCE WITH THE DRAWINGS. A COPY OF THE DEFERRED SUBMITTAL DOCUMENTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THEIR DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.
- 4. DEFERRED SUBMITTAL ITEMS:

WOOD TRUSSES

REVIEWED FOR STRUCTURAL CODE





JOHNSON RESIDENCE 15-3A RESOLUTION POINTE ANCHORAGE, ALASKA



THE ENGINEER IS RESPONSIBLE ONLY FOR THE WORK ON THOSE SHEETS BEARING THE ENGINEER'S STAMP. THESE DOCUMENTS MAY NOT BE REPRODUCI IN ANY FORM WITHOUT THE EXPRESS WRITTE CONSENT OF SCHNEIDER & ASSOCIATES STRUCTURAL ENGINEERS INC. SCHNEIDER & ASSOCIATES STRUCTURAL ENGINEERS INC. IS NOT RESPONSIBLE FOR ANY REVISIONS OR ADDITIONS TO THESE DRAWINGS UNLESS INITIALED OR AGREED TO IN WRITING BY THE ENGINEER.

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revisions number date

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06/17/15 51.2

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A OR G DIMENSION DIMENSION 180° 90°

	100	90				
END HOOKS,ALL GRADES						
BAR	FINISHED	180-DEG H	HOOKS	90 DEG HOOKS		
SIZE	BEND DIA. D,IN.	A OR G, IN.	J, IN.	A OR G,IN.		
#3	2.25	5	3	6		
#3 #4 #5 #6 #7	3	6	4	8		
#5	3.75	7	5	10		
#6	4.5	8	6	12		
#7	5.25	10	7	14		
#8 #9	6	11	8	16		
#9	9.5	15	11.75	19		
#10	10.75	17	13.25	22		
#11	12	19	14.75	24		
#14	18.25	27	21.75	31		
#18	24	36	28.5	41		

	FOOTING OR FOUNDATION. 3. BOTTOM OF CONCRETE FOOTING. 4. BOTTOM OF TRENCH. 5. 30° MAX IN GRANULAR SOILS, 45° MAX IN OTHER SOILS U.N.O.
(3) — (2)	⇔ SHORE ⇒
(5)	NOTE: FOR ADDITIONAL INFORMATION, SEE PLANS AND DETAILS.

TYPICAL TRENCH PARALLEL TO FOUNDATION

MIN.

SCALE: NOT TO SCALE

NOTES:

NOTES:

1. LAP PER G.S.N. (24" MIN.)

PIPE THRU STEM WALL AS

OCCURS - SEE TYPICAL

THICKNESS AS SHOWN ON

2. TOP OF WALL FOOTING.

4. RETURN TO FOOTING

TYPICAL.

DETAIL.

PLAN.

1. FINISHED GRADE WHERE

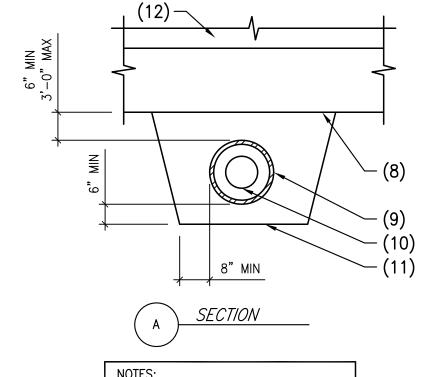
2. DO NOT EXCAVATE A TRENCH

BELOW IMAGINARY LINE

EXTENDING FROM BOTTOM OF

200-006-TYP

OCCURS.



A. NO PIPE SHALL PASS THROUGH FOOTINGS OR UNDER COLUMN FOOTINGS.

B. FOR TRENCHES GREATER THAN 3'-0" BELOW BOTTOM OF FOOTING SEE TYPICAL PIPE PASSING BELOW WALL FOOTING DETAIL.

10'-0"

9. SLEEVE. PROVIDE 1/2" MINIMUM CLEARANCE AROUND PIPE OR CONDUIT. 10. PIPE OR CONDUIT. 11. CONCRETE FILL TO BE PLACED BEFORE FOOTING IS POURED. FORM SAME AS FOOTING AND

> POUR FULL WIDTH OF PIPE TRENCH. 12. FOUNDATION WALL.

NOTES:

1. CAST IRON SLEEVE 2"

LARGER THAN PIPE.

2. NO SLEEVES PERMITTED

THROUGH FOOTING.

EXCAVATION BELOW

THESE LINES NOT

TRENCH. EXTEND 2'-0"

EACH SIDE OF SLEEVE.

FOOTING IF NECESSARY

IN ORDER TO MAINTAIN

90% DENSITY (ASTM D-698).

3'-0" MAXIMUM. STEP

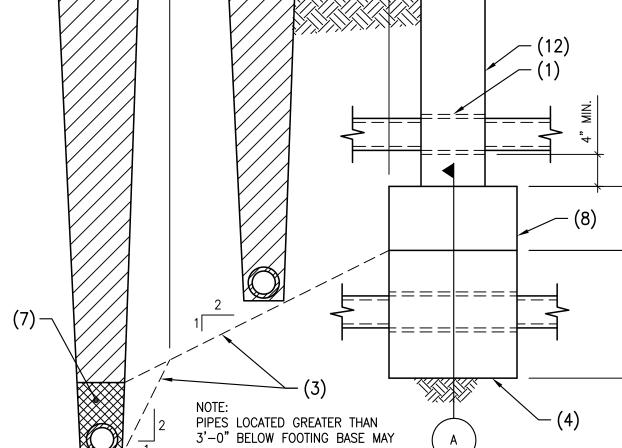
3'-0" MAX HEIGHT. BACKFILL PER SPECS.

CONCRETE FOOTING.

PERMITTED.

4. CONCRETE FILL PIPE

1'-6" ===|=== :===|=== PIPES LOCATED GREATER THAN



BE BACKFILLED WITH COMPACTED FILL PER SPECIFICATIONS

TYPICAL PIPE THROUGH FOOTING AND TRENCH

NOTES: STEM WALL. CONCRETE FOOTING. 1'-6" MAXIMUM - WHERE TRENCH EXCEEDS 1'-6", NOTIFY STRUCTURAL ENGINEER PRIOR TO PLACEMENT OF FOOTING. 4. BACKFILL AND RECOMPACT

TRENCH PER SOILS REPORT AND SPECIFICATIONS. BOTTOM OF TRENCH.

> FOR TRENCH DEPTH OF 3'-6" OR LESS, SEE TYPICAL PIPE AT FOOTING DETAIL.

200-005-TYP

NO PIPE SHALL PASS THRU FOOTINGS OR UNDER COLUMN FOOTINGS.

TYPICAL PIPE PASSING BELOW WALL FOOTING IN DEEP TRENCH

| # | 8 | <u> 180° HOOK (#5 BAR)</u>

NOTES:

MAX

TYPICAL HOLDOWN ANCHOR AT CONCRETE FOUNDATION WALL

REINFORCING

12"

MECHANICAL

2 3/4"

3 1/2"

4 1/2"

5 1/2"

ANCHOR EMBEDMENT ANCHOR EMBEDMENT

ENGTH IN CONCRETE LENGTH IN MASONRY

PLATE, ANGLE

STEEL EMBEDMENT

ENGTH IN CONCRETE LENGTH IN MASONRY

MAX 🛦

SCALE: NOT TO SCALE

REINFORCING

STEEL SIZE

#3

ANCHOR

DIAMETER

3/8"

1/2"

5/8"

3/4"

7/8"

1 1/4"

FACE OF CONCRETE

/ || \

MECHANICAL ANCHOR

(NOTE 4)

OR MASONRY ·

REINFORCING

STEEL EMBEDMENT

10**"**

12"

MECHANICAL

5 1/4"

5 3/4"

1. THREADED ROD HOLDOWN ANCHOR

WITH 3/16" THICKx2" DIA.

NUTS AT EMBEDDED END.

#5 FOOTING DOWEL TO LAP

CONCRETE FOUNDATION WALL.

COUPLER NUT AS REQUIRED.

ENGAGE FOOTING REINFORCING

BELOW FOR QUANTITY.

FOOTING REINFORCING.

WITH FOOTING DOWEL.

THREADED ROD HOLDOWN

ANCHOR DIA.

1 1/4"

NOTES:

CONCRETE FOOTING.

WITH HOOKED BAR. DOWEL QUANTITY

TO MATCH HOOKED BAR QUANTITY.

#5 x 48" LONG WITH 180° HOOK

ÄT HOLDOWN ANCHOR. SEE SCHEDULE

602-018-TYP

1. PROVIDE POST-INSTALLED ANCHORS

SCHEDULE UNLESS NOTED ON

POST-INSTALLED ANCHORS SHALL

APPLY TOWARDS EMBEDMENT.

ARE NOT LIMITED TO WEDGE.

UNDERCUT AND SCREW TYPE

THICKNESS OF DRYPACK DOES NOT

MECHANICAL ANCHORS INCLUDE BUT

THREADED

ROD ANCHOR

EMBEDMENT LENGTH

IN MASONRY

3 1/2"

4 1/2"

PLANS OR DETAILS.

ANCHORS.

THREADED

EMBEDMENT LENGTH

ROD ANCHOR

IN CONCRETE

4 1/2"

6 3/4"

6 3/4"

10"

HAVE I.C.C. APPROVAL.

AND REINFORCING STEEL PER THIS

#5 HOOKS

PLATE WASHER AND DOUBLE

TYPICAL REINFORCING HOOK SCHEDULE

SCALE: NOT TO SCALE 200-045-TYP

SPLICE DETAIL

6 MIN BAR OFFSET \vdash

BAR CLEARANCE

1. LAP - SEE G.S.N. 2. MAXIMUM 1/5 LAP BUT NOT MORE THAN 6".

WIRE TIES. 4. 1d (1" MINIMUM).

RADIUS=3d FOR BARS NOT OVER #8; 4d FOR #9, #10 AND #11 BARS; 5d FOR #14 AND #18 BARS. 5d FOR ALL GRADË 40 BARS WITH 180 DEGREE HOOK.

6. 4d (2 1/2" MINIMUM). 12d (90 DEGREE HOOK).

BEND AROUND 1 1/2" PIN FOR #3 BARS. BEND AROUND 2" PIN FOR #4 BARS. BEND AROUND

200-050-TYP

1. PROVIDE ANCHORS, ANCHOR BOLTS,

AND HEADED STUDS PER THIS

2. SCHEDULE APPLIES TO ANCHORS

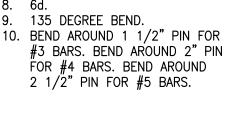
IN CONCRETE AND MASONRY.

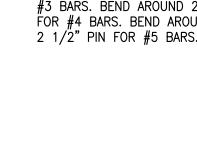
APPLY TOWARDS EMBEDMENT.

3. THICKNESS OF DRYPACK DOES NOT

OR DETAILS.

SCHEDULE UNLESS NOTED ON PLANS





BEAM STIRRUPS COLUMN TIES

BEND & HOOK DETAILS

7	TYF	PICAL	CON	ICRETE	RE	NFORC	ING	BAR	DETA	A/LS	
	SCALE	E: NOT	TO S	CALE							
		VEDT		1.10017	DOLE		OTU	$\overline{}$		NOTES:	

ANCHOR DIAMETER	VERT BOLT EMBEDMENT LENGTH	HORIZ BOLT EMBEDMENT LENGTH	HEADED STUD FILLET WELD SIZE, "S"
1/2"	7"	4"	1/4"
5/8"	7"	4"	5/16"
3/4"	7"	5 "	5/16"
7/8"	8"	6 "	5/16"
1"	9"	7"	3/8"
1 1/8"	10"	8"	
1 1/4"	11"	9"	

	., -		•				
	1"	9"	7"	3/8"			
	1 1/8"	10"	8"				
	1 1/4"	11"	9"				
CH	ATE, ANGLE ANNEL, ETC OTE 3)	s.		FACE OF CON OR MASONRY		PLATE, ANGLE CHANNEL, ETC. (NOTE 3)	
	EMBEDMENT	3/	EMBEDMENT	2" MIN.	STANDARD HOOK OR BOLT HEAD —TYPICAL	EMBEDMENT	

CHAN (NOTI	INEL, ETC. E 3) —			1 1/8"	10"	8"	
•		, I	<u></u>	1 1/4"	11"	9"	
	E = = = =	E ≡ ≡ ≡ ≡ ≡	СН	ATE, ANGLE ANNEL, ETC OTE 3)			FAC OR
<u>IOR</u>	ADHESIVE ANCHOR (THREADED ROD OR REINFORCING STEEL)	ADHESIVE ANCHOR (THREADED ROD OR REINFORCING STEEL)		EMBEDMENT	3/	EWBEDWENT 191	2" N

501-002-TYP

AND HEADED STUD SCHEDULE

<u>ANCHOR</u>

501-001-TYP

HEADED STUD

CONCRETE FOOTING AND/OR STEM WALL SCALE: NOT TO SCALE

TYPICAL CORNER REINFORCING IN

<u>CORNER</u>

COMPLIANCE

TYPICAL CAST-IN-PLACE ANCHOR, ANCHOR BOLT, SCALE: NOT TO SCALE

ANCHOR BOLT

NOTES: CORNER BARS SAME SIZE AND SPACING AS HORIZONTAL (24" MINIMUM). FOOTING.

CONCRETE STEM WALL OR 4. REINFORCING PER PLANS AND /OR G.S.N.

REVIEWED FOR STRUCTURAL CODE

SCALE: NOT TO SCALE

TYPICAL STEP IN CONCRETE FOOTING SCALE: NOT TO SCALE

2D 2D

2D

REINFORCING. LAP PER G.S.N. ALTERNATE BEND.

D = 2'-0" MAXIMUM.

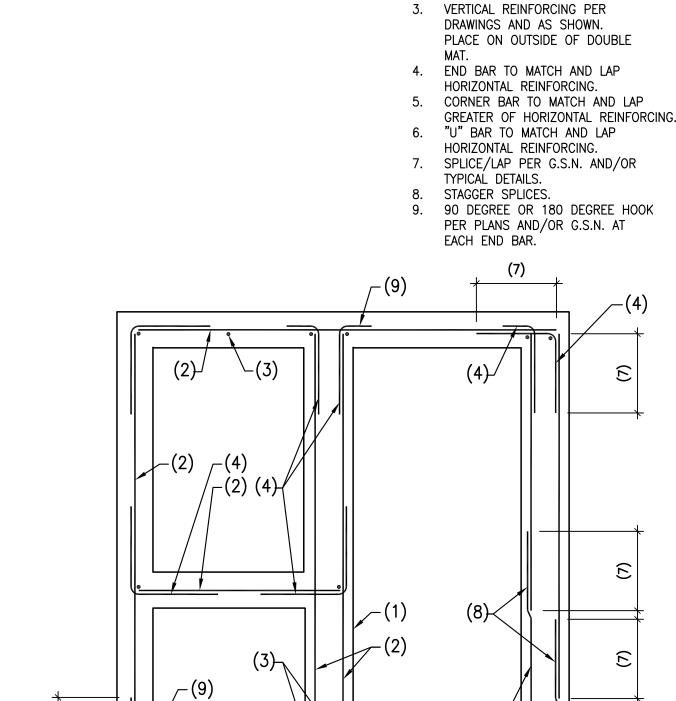
200-020-TYP

(2) (3)

SCALE: NOT TO SCALE

TYPICAL POST—INSTALLED ANCHOR AND REINFORCING STEEL SCHEDULE

200-030-TYP



(4)

TYPICAL REINFORCING IN CONCRETE WALL

(7)

NOTES:

1. CONCRETE WALL.

HORIZONTAL REINFORCING.

NOTE: CONTRACTORS OPTION TO PROVIDE STANDARD HOOKS ON HORIZONTAL REINFORCING AS SHOWN IN LIEU OF 90 DEGREE BENT END AND/OR CORNER BARS.

(7)

SCALE: NOT TO SCALE 215078-S1.0-16 EDGE ATTACHMENT. 2. WOOD TRUSS. NOTE: TRUSS MANUFACTURER TO PROVIDE VERTICAL WEB TO MATCH LOCATION OF SHEAR 3. 7/16" PLYWOOD SHEAR PANEL WITH 8d AT 6" O.C. AT PANEL EDGES. ORIENT FACE GRAIN VERTICALLY. 4. 16d AT 6" O.C. TO TRUSS VERTICAL WEB. $\overline{\circ}$ 16d TO MATCH "EDGE ATTACHMENT" SPACING. PLYWOOD SHEATHING. 2x4 AT FOUR SIDES. TOP OF WOOD PLATE OR BEAM AS OCCURS. DRILLED VENTILATION HOLES PER ARCHITECTURAL.

FOR CONSTRUCTION BELOW PANEL, SEE PLAN.

TYPICAL PLYWOOD SHEAR PANEL SCALE: NOT TO SCALE

215078-S1.0-17

WOOD STUD. 2. CUT OR NOTCH. OVERCUTTING IS NOT PERMITTED. NOTCH IS PERMITTED AT THE TOP OF THE MEMBER, NOT FURTHER FROM THE FACE OF THE SUPPORT THAN THE DEPTH OF THE MEMBER. CUTS OR NOTCHES ARE PERMITTED ONLY IN HATCHED L/3 L/3 L/3 5. WOOD JOIST OR BEAM.

JOISTS AND BEAMS 0.25W 0.40W BEARING WALL STUDS NON-BEARING WALL STUDS

OR NOTCHES ARE NOT PERMITTED. B. "NON-BEARING" REFERS TO WALLS SUPPORTING NO LOADS OTHER THAN THE WEIGHT OF THE WALL ALL OTHER WALLS SHALL BE CONSIDERED "BEARING" C. THIS DETAIL APPLIES TO SOLID SAWN LUMBER ONLY, NOT TO ENGINEERED WOOD PRODUCTS OR GLULAMS. D. CONTACT ENGINEER IF CONDITIONS EXCEED THOSE

WOOD STUD WALL.

BOTH SIDES.

16d AT 12" O.C. STAGGERED

(2) 16d AS SHOWN. (2) 16d TOENAILS — EACH

RUN VERTICAL STUD UP PAST LINTEL AS SHOWN. SEE JAMB STUD FOR EXTERIOR OPENINGS.

DOUBLE STUDS UNDER LINTEL BEARINGS FOR OPENING WIDTHS GREATER THEN

SIDE - EACH END.

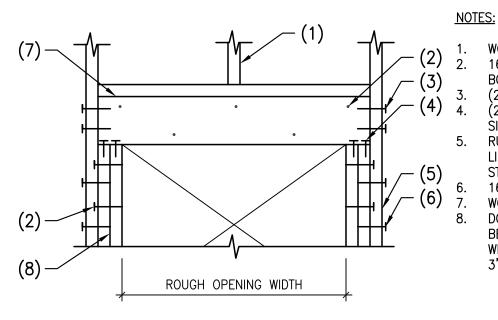
16d AT 12" O.C.

WOOD LINTEL.

3'-0"

NOTE:
A. AT EXTERIOR WALLS, CUTS

TYPICAL CUTS AND NOTCHES IN SOLID SAWN WOOD FRAMING SCALE: NOT TO SCALE 601-007-TYP

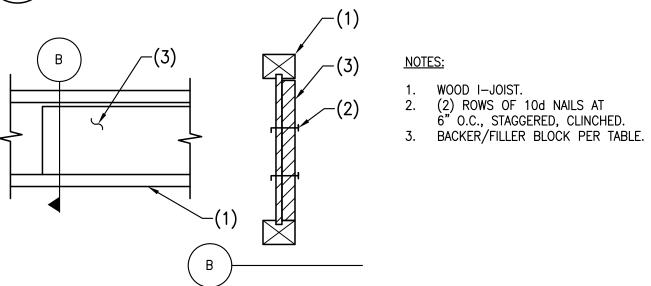


ROUGH OPENING WIDTH	LINTEL SIZE	JAMB
UP TO 3'-0"	(3) 2x8	(2) 2x6
3'-1" TO 5'-0"	(3) 2x10	(2) 2x6
5'-1" TO 7'-0"	(3) 2x12	(2) 2x6

THIS SCHEDULE MAY BE USED ONLY IN WALLS WHERE NO OTHER LINTEL HAS BEEN CALLED OUT ON THE PLANS OR SCHEDULES.

601-002-TYP

TYPICAL NON-SCHEDULED LINTEL SCALE: NOT TO SCALE



SERIES	BACKER BLOCK THICKNESS	FILLER BLOCK THICKNESS
5000 1.7	3/4" OR 7/8" WOOD PANELS	TWO 3/4" WOOD PANELS OR 2x_
6000 1.7	1 1/8" OR TWO 1/2" WOOD PANELS	2x +5/8" OR 3/4" WOOD PANEL
6500 1.8	WOOD PANELS	2x +5/8" OR 3/4" WOOD PANEL
60 2.0	1 1/8" OR TWO 1/2" WOOD PANELS	2x +5/8" OR 3/4" WOOD PANEL
90 2.0	2x LUMBER	DOUBLE 2x LUMBER

CUT BACKER AND FILLER BLOCKS TO A MAXIMUM DEPTH EQUAL TO THE WEB DEPTH MINUS 1/4" TO AVOID A FORCED FIT. EXTEND WEB FILLER 4" PAST.

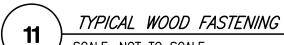
TYPICAL BOISE WOOD I—JOIST WEB BACKER/FILLER DETAIL 215078-S1.0-15 SCALE: NOT TO SCALE

REVIEWED FOR STRUCTURAL CODE COMPLIANCE



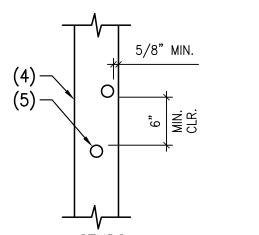
CONNECTION FASTENING		LOCATION
JOIST TO SILL OR GIRDER	(3) 8d COMMON (2 1/2"x0.131") (3) 3"x0.131" NAILS	TOENAIL
BRIDGING TO JOIST	(2) 8d COMMON (2 1/2"x0.131") (2) 3"x0.131" NAILS	TOENAIL EACH END
SILL PLATE TO JOIST OR BLOCKING	16d (3 1/2"x0.135") AT 16" 0.C. 3"x0.131" NAILS AT 8" 0.C.	TYPICAL FACE NAIL
TOP PLATE TO STUD	(2) 16d COMMON (3 1/2"x0.162") 3-3"x0.131" NAILS	END NAIL
STUD TO SILL	(4) 8d COMMON (2 1/2"x0.131") (4) 3"x0.131" NAILS	TOENAIL
PLATE	(2) 16d COMMON (3 1/2"x0.162") (3) 3"x0.131" NAILS	END NAIL
BUILT-UP STUDS	16d (3 1/2"x0.135") AT 12" O.C. 3"x0.131" NAILS AT 8" O.C.	FACE NAIL
DOUBLE TOP	16d (3 1/2"x0.135") AT 16" O.C. 3"x0.131" NAILS AT 12" O.C.	TYPICAL FACE NAIL
PLATES	(8) 16d COMMON (3 1/2"x0.162") (12) 3"x0.131" NAILS	LAP SPLICE
BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	(3) 8d COMMON (2 1/2"x0.131") (3) 3"x0.131" NAILS	TOENAIL
RIM JOIST TO TOP PLATE	8d (2 1/2"x0.131") AT 6" O.C. 3"x0.131" NAIL AT 6" O.C.	TOENAIL
TOP PLATES, LAPS AND INTERSECTIONS	(2) 16d COMMON (3 1/2"x0.162") (3) 3"x0.131" NAILS	FACE NAIL
CONT. HEADER, TWO PIECES	16d COMMON (3 1/2"x0.162")	16" O.C. ALONG EDGE
CONT. HEADER TO STUD	(4) 8d COMMON (2 1/2"x0.131")	TOENAIL
WOOD JOIST TO PLATE	(3) 8d COMMON (2 1/2"x0.131") (3) 3"x0.131" NAILS	TOENAIL
BUILT-UP CORNER STUDS	16d COMMON (3 1/2"x0.162") 3"x0.131" NAILS	24" O.C. 16" O.C.
BUILT-UP GIRDER	20d COMMON (4"x0.192") 32" O.C. 3"x0.131" NAIL AT 24" O.C.	FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES
AND BEAMS	(2) 20d COMMON (4"x0.192") (3) 3"x0.131" NAILS	FACE NAIL AT ENDS AND AT EACH SPLICE

1.COMMON OR BOX NAILS ARE PERMITTED TO BE USED EXCEPT WHERE OTHERWISE STATED. 2.STAPLES SHALL HAVE A MINIMUM CROWN OF 7/16". 3. FASTENING SCHEDULE APPLIES U.N.O., WHERE DIFFERENCES OCCUR, GREATER REQUIREMENT



GOVERNS.

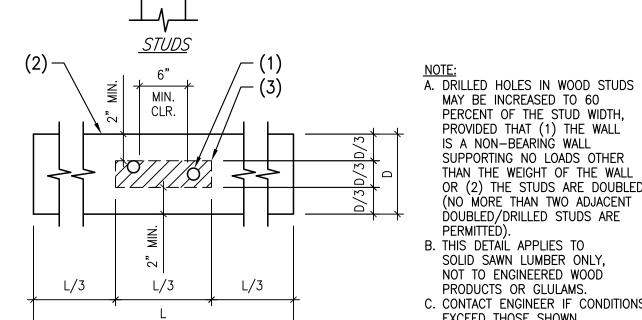
SCALE: NOT TO SCALE 601-008-TYP



NOTES: DRILLED HOLE DIAMETER. NOT TO EXCEED 1/3 OF THE JOIST/BEAM DEPTH.

WOOD JOIST OR BEAM. DRILLED HOLES ARE PERMITTED ONLY IN HATCHED AREA.

WOOD STUD. DRILLED HOLE DIAMETER. NOT TO EXCEED 40 PERCENT OF STUD WIDTH (SEE NOTE).



SCALE: NOT TO SCALE

MAY BE INCREASED TO 60 PERCENT OF THE STUD WIDTH, PROVIDED THAT (1) THE WALL IS A NON-BEARING WALL SUPPORTING NO LOADS OTHER THAN THE WEIGHT OF THE WALL OR (2) THE STUDS ARE DOUBLED (NO MORE THAN TWO ADJACENT DOUBLED/DRILLED STUDS ARE PERMITTED). B. THIS DETAÍL APPLIES TO

SOLID SAWN LUMBER ONLY, NOT TO ENGINEERED WOOD PRODUCTS OR GLULAMS.

C. CONTACT ENGINEER IF CONDITIONS EXCEED THOSE SHOWN.

JOISTS AND BEAMS TYPICAL DRILLED HOLES IN SOLID SAWN WOOD FRAMING

601-006-TYP

JOHNSON RESIDENCE 15-3A RESOLUTION POINTE ANCHORAGE, ALASKA

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project 215078 engineer W. OVIATT

drafter R. BRANCH 06/17/15

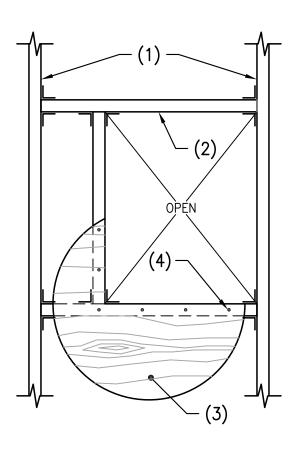
51.3 sheet

NOTES:

- WOOD TRUSS OR I-JOIST.
 TYPICAL: 2x6 WITH SIMPSON LUS26 HANGER OR (2) SIMPSON A35 CLIPS.
 PLYWOOD ROOF SHEATHING.
 RE-ATTACH EXISTING SHEATHING WITH 10d NAILS AT 6" O.C.

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JOHNSON RESIDENCE 15-3A RESOLUTION POINTE ANCHORAGE, ALASKA



TYPICAL FRAMED OPENING IN PLYWOOD ROOF/FLOOR SHEATHING SCALE: NOT TO SCALE 1. 2x6 AT 24" O.C., MAXSPAN = 6'-0". WOOD TRUSS BELOW. 4. 2x8 LAID FLAT OVER SHEATHING WITH TYPICAL. NOTES:

A. PLYWOOD SHEATHING OVER
PRIMARY ROOF TRUSSES
SHALL BE CONTINUOUS.

B. FOR CLARITY, SHEATHING

TYPICAL BASE PLATE

5. 2" MINIMUM OR AS REQUIRED FOR WRENCHING CLEARANCE. NOTE:
PROVIDE TEMPORARY SUPPORT
DURING ERECTION FOR
COLUMNS WITH 2 BOLT BASE
PLATES.

OFFSET BASE PLATE

SCALE: NOT TO SCALE

<u> 2 BOLT BASE PLATE</u>

TYPICAL STEEL COLUMN BASE PLATE

505-003-TYP

S1.4

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215078-S1.0-20 LINE OF SHEATHING JOINTS. (3) 10d AT EACH TRUSS. SIMPSON A34 EACH SIDE —

PLAN - TYPICAL VALLEY OVERFRAMING AT WOOD TRUSSES

NOT SHOWN.

OVER VALLEY FRAMING

215078-S1.0-21 1. CENTERLINE OF COLUMN AND STEEL BASE PLATE. 2. STEEL BASE PLATE.

3. 1 1/4" MINIMUM OR PER A.I.S.C. TABLE J3.4.

4. STEEL COLUMN – FOR TYPE,
SIZE, BASE PLATE AND ANCHOR BOLTS, SEE SCHEDULE.

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- A. EXTEND TOP AND BOTTOM REINFORCING 24" MINIMUM BEYOND FACE OF CONCRETE EACH SIDE. AT CORNERS, EXTEND BARS AROUND CORNER AT WALL ENDS, TERMINATE WITH 90 OR 180 DEGREE HOOK.

 B. WHERE ACTUAL HEADER DEPTH EXCEEDS TWO TIMES THE SCHEDULED MINIMUM HEADER DEPTH, STIRRUP REINFORCING IS NOT REQUIRED.

1	HEADEN DEI III, STIMOI MEINI ONGINO 13 NOT MEGOINED.							
MARK	ROUGH OPENING WIDTH	MINIMUM HEADER DEPTH	TOP AND BOTTOM REINFORCING	STIRRUP REINFORCING				
WO1	UP TO 3'-0"	10"	(1) #5 T & B					
WO2	3'-1" TO 8'-0"	10"	(2) #5 T & B	(2) #3 AT 8" O.C.				
WO3	8'-1" TO 13'-6"	12"	(2) #5 TOP (2) #7 BOTTOM	(2) #3 AT 8" O.C.				

- 1. I.C.F. WALL. 2. TYPICAL ALL OPENINGS: HALF OF WALL REINFORCEMENT REMOVED AT OPENING TO BE PLACED EACH SIDE OF OPENING. USE (2) #5 BARS MINIMUM EACH SIDE TOP AND BOTTOM.
- 3. TOP OF WALL OR COLD JOINT.
- BOTTOM REINFORCING.
- SHEAR REINFORCING WITH 180 DEGREE HOOK EACH END. USTART SHEAR REINFORCING AT FACE OF ROUGH OPENING AT EACH JAMB.
- FACE OF CONCRETE. 8. TREATED 2x LUMBER BUCK.

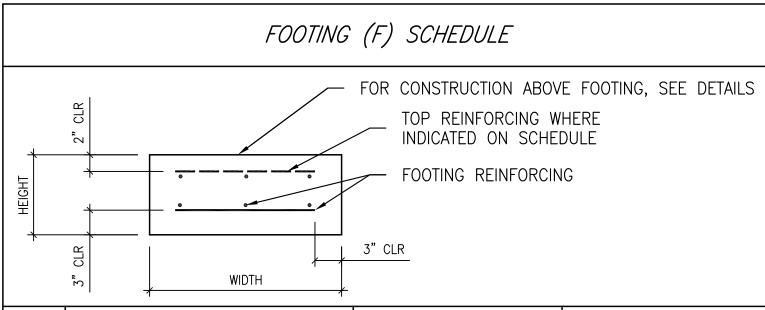
LEDGER (L) SCHEDULE						
MARK	TYPE	SIZE	CONNECTION	REMARKS		
L1	WOOD	2x12	SIMPSON ICVL WITH ICVL—CW AT 48" O.C. WITH (8) ICF—D3.62 SCREWS			
L2	WOOD	2x8	SIMPSON ICVL WITH ICVL—CW AT 48" O.C. WITH (8) ICF—D3.62 SCREWS			

1. ALL LEDGERS SHALL HAVE MINIMUM OF 2 ANCHORS/FASTENERS PER LEDGER PIECE.
2. ANCHORS/FASTENERS SHALL BE LOCATED NOT LESS THAN 6" NOR MORE THAN 12" FROM END OF LEDGER PIECES OR AT LEDGER SPLICES.

	JOIST (J) SCHEDULE							
MARK	TYPE/SIZE	JOIST SPACING	END CONNECTION	REMARKS				
J1	11 7/8" BCI 6000	16" O.C.	SIMPSON IUS OR ITS HANGER	 				
J2	2x10	16" O.C.	SIMPSON LUS HANGER	 				

	WOOD TRUSS (T) SCHEDULE						
MARK	CONFIGURATION	SPACING	REMARKS				
<i>T1</i>		24" O.C. U.N.O.	SEE NOTE 1				
72	$\hat{\psi}$	24" O.C. U.N.O.	SEE NOTE 1				
73	\(\frac{1}{\dot}\)	24" O.C. U.N.O.	SEE NOTE 1				
<i>T4</i>	$\hat{\nabla}$	24" O.C. U.N.O.	SEE NOTE 1				
75		24" O.C. U.N.O.	SEE NOTE 1				
76		24" O.C. U.N.O.	SEE NOTE 1				
77	₹ Property of the contract of	24" O.C. U.N.O.	SEE NOTE 1				
78	\(\frac{1}{\chi}\)	24" O.C. U.N.O.	SEE NOTE 1				
79	$\hat{\psi}$	24" O.C. U.N.O.	SEE NOTE 1				
NOTES	•						

- SEE ARCHITECTURAL DRAWINGS FOR REQUIRED DIMENSIONS, DEPTHS, PROFILES, ETC. TRUSS FABRICATOR TO PROVIDE OVERFRAMING AT AREAS SHOWN ON PLANS. AT FABRICATORS OPTION, TRUSSES MAY BE FABRICATED TO INCLUDE OVERFRAMING (PIGGY BACK) INTO TRUSS CONFIGURATION.
- TRUSS GIRDÉR DESIGNATED AS 'TG#'.
- TRUSSES SHALL BE DESIGNED FOR AN ADDITIONAL 300 LBS CONCENTRATED DEAD LOAD AT ANY POINT.



MADIZ		DIMENSIONS		FOOTING	REMARKS
MARK	HEIGHT	WIDTH	LENGTH	REINFORCING	KEMAKNS
F1	10"	2'-0"	CONT.	(3) #5 BARS CONT.	
F2	10"	1'-8"	CONT.	(3) #5 BARS CONT.	
F3	10"	1'-4"	CONT.	(2) #5 BARS CONT.	
F4	10"	3'-0"	3'-0"	(4) #5 BARS, EACH WAY	
F5	10"	4'-0"	4'-0"	(5) #5 BARS, EACH WAY	
F6	12"	6'-0"	6'-0"	(7) #5 BARS, EACH WAY	

1. FOR FOOTING BEARING DEPTH BELOW GRADE, SEE G.S.N. U.N.O. 2. CENTER FOOTINGS UNDER WALLS OR COLUMNS U.N.O.

MARK	SIZE	BASE CONNECTION	REMARKS
C1	6x6	SIMPSON ABU66 U.N.O.	
C2	3" XXSTRONG STEEL PIPE	(4) 5/8" DIA. C.I.P. ANCHOR BOLTS SEE TYPICAL DETAILS FOR EMBEDMENT AND BASE PLATE GEOMETRY	

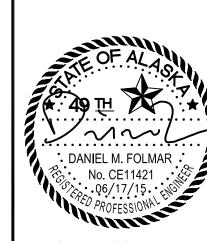
	BEAM (B) SCHEDULE					
MARK	SIZE	END CONNECTION	REMARKS			
B1	4x12	PER PLAN/DETAILS				
B2	5 1/8" x 12" GLB	PER PLAN/DETAILS				
В3	5 1/8" x 16 1/2" GLB	PER PLAN/DETAILS				
<i>B4</i>	8 3/4" x 16 1/2" GLB	PER PLAN/DETAILS				

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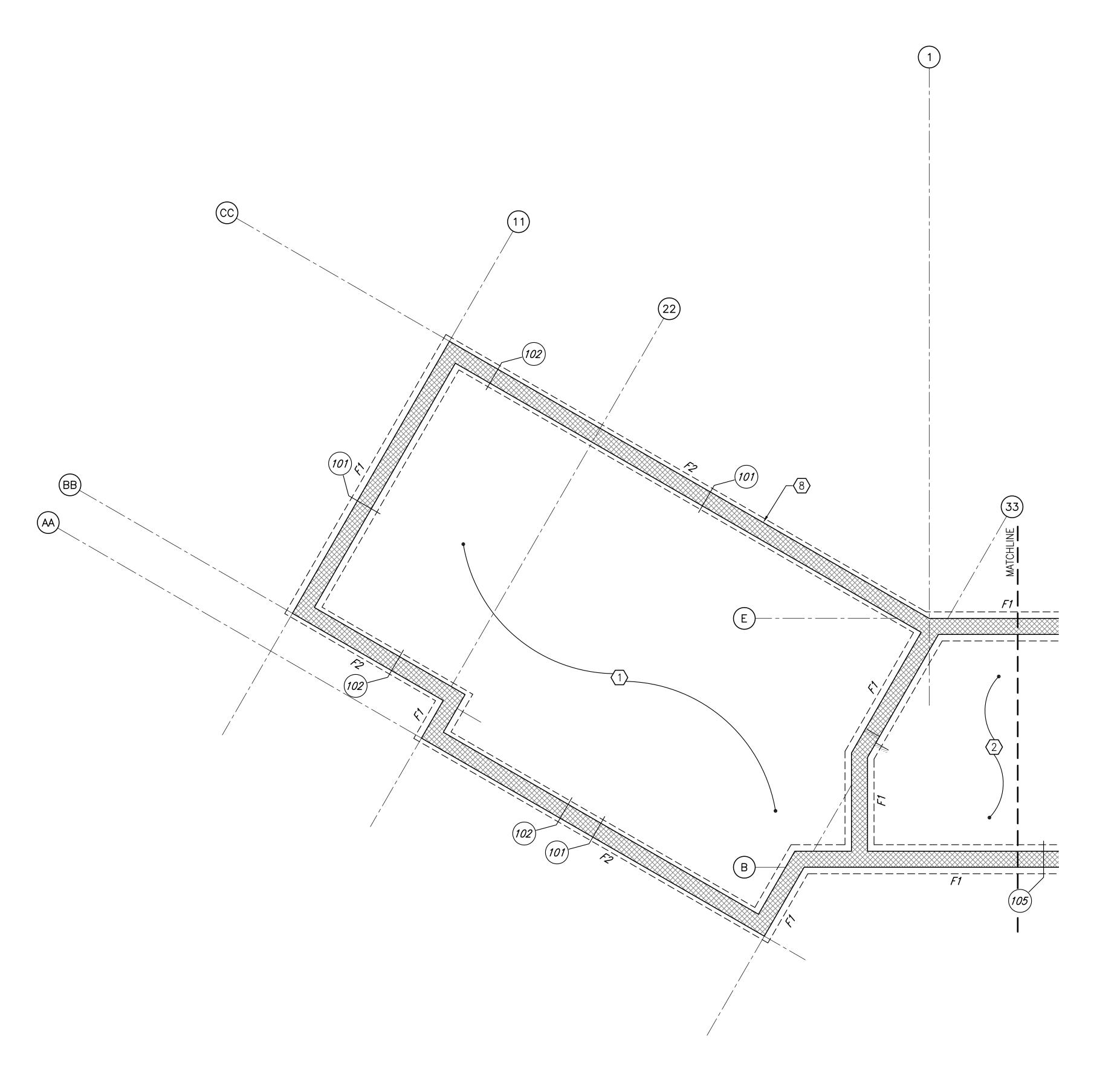


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project 215078

engineer W. OVIATT drafter R. BRANCH 06/17/15

S1.5



FOUNDATION KEYNOTES:

- 1 UNEXCAVATED NATIVE SOIL OR FILL.
- (2) HEATED CRAWL SPACE.
- TYPICAL AS INDICATED: WOOD COLUMN.
- 4 WOOD STUD PONY WALL.
- TYPICAL AS INDICATED: 3" DIA. MINI PILE, PER MOA POLICY. DESIGN LOAD = 4.9 KIPS.

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- 6 UNHEATED CRAWL SPACE.
- 5 STEP FOOTING AS REQUIRED TO MAINTAIN FROST PROTECTION PER G.S.N.
- TYPICAL U.N.O.: 8" I.C.F. CONCRETE WALL. REINFORCE WITH #5 VERTICALS AT 16" O.C. AND #5 HORIZONTALS AT 16" O.C., CENTERED IN WALL. SEE "TYPICAL REINFORCING IN CONCRETE WALL" DETAIL. AT WALL ENDS AND OPENINGS, INSTALL 180 DEGREE HOOK. AT CORNERS, INSTALL 90 DEGREE REINFORCING.

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JOHNSON RESIDENCE
15-3A RESOLUTION POINTE
ANCHORAGE, ALASKA
FOUNDATION PLAN



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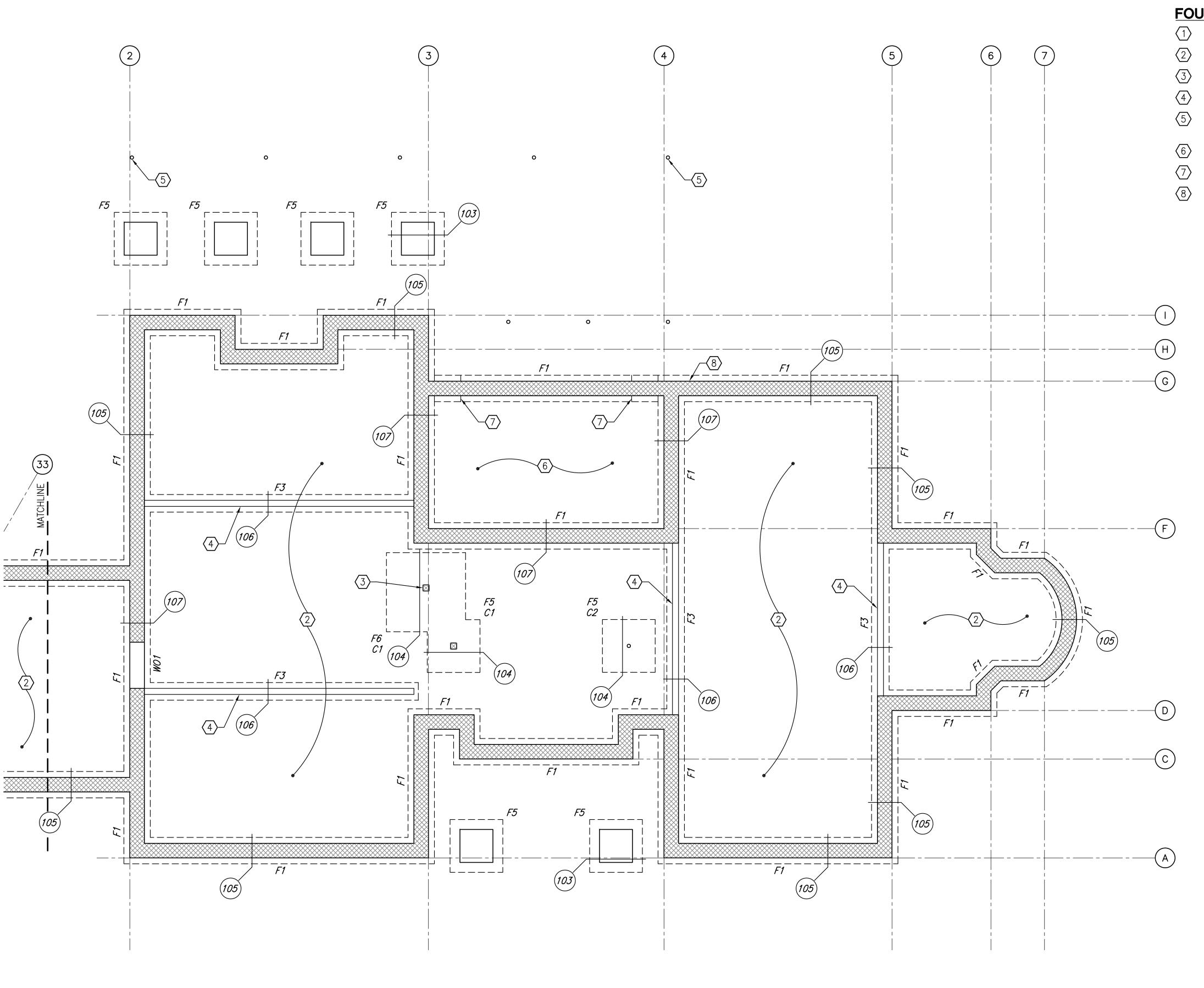
<u>project</u> 215078 <u>engineer</u> W. OVIATT

<u>drafter</u> R. BRANCH date 06/17/15

\$2.0

FOUNDATION PLAN

SCALE: 1/4"=1'-0"



FOUNDATION KEYNOTES:

1 UNEXCAVATED NATIVE SOIL OR FILL.

 $\overline{2}$ HEATED CRAWL SPACE.

3 TYPICAL AS INDICATED: WOOD COLUMN.

4 WOOD STUD PONY WALL.

TYPICAL AS INDICATED: 3" DIA. MINI PILE, PER MOA POLICY. DESIGN LOAD = 4.9 KIPS.

6 UNHEATED CRAWL SPACE.

 $\overline{7}$ STEP FOOTING AS REQUIRED TO MAINTAIN FROST PROTECTION PER G.S.N.

TYPICAL U.N.O.: 8" I.C.F. CONCRETE WALL. REINFORCE WITH #5 VERTICALS AT 16" O.C. AND #5 HORIZONTALS AT 16" O.C., CENTERED IN WALL. SEE "TYPICAL REINFORCING IN CONCRETE WALL" DETAIL. AT WALL ENDS AND OPENINGS, INSTALL 180 DEGREE HOOK. AT CORNERS, INSTALL 90 DEGREE REINFORCING.

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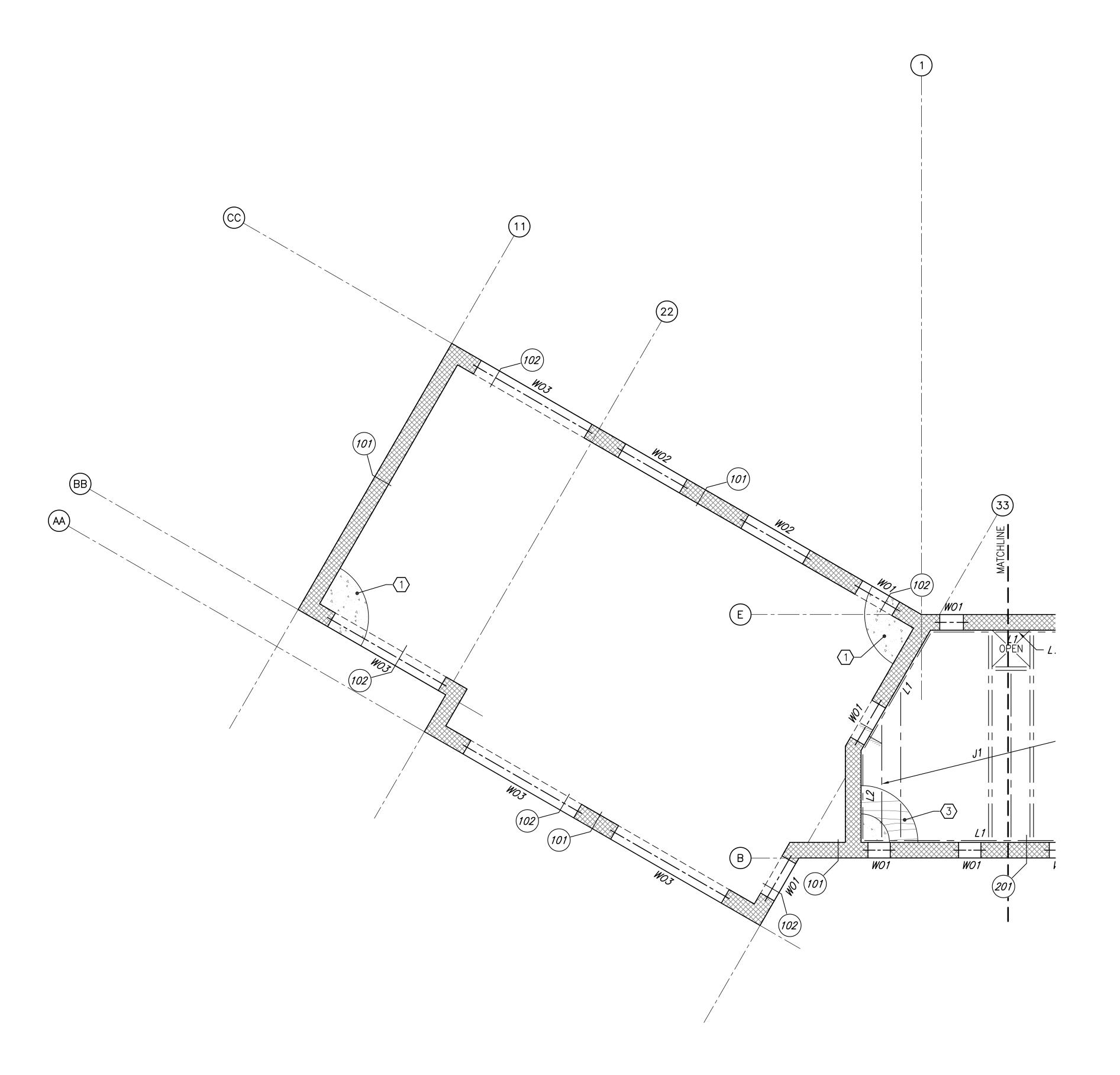
 engineer
 W. OVIATT

 drafter
 R. BRANCH

 date
 06/17/15

S2.1

FOUNDATION PLAN
SCALE: 1/4"=1'-0"



FRAMING KEYNOTES:

- 4" CONCRETE SLAB ON GRADE. REINFORCE WITH 6x6 W1.4-W1.4 W.W.F. 1 1/2" CLEAR FROM TOP OF SLAB.
- (2) WOOD STAIRS PER G.S.N.
- 1 1/2" GYPCRETE OVER FLOOR SHEATHING PER G.S.N.
- SIMPSON HGU BEAM HANGER.
- TYPICAL: SIMPSON CC TYPE WELDED COLUMN CAP AT BEAMS CONNECTION TO MINI PILES.
- TYPICAL: SIMPSON ECC TYPE WELDED COLUMN CAP AT END BEAMS CONNECTING TO MINI PILES.
- 7 DOUBLE FLOOR JOISTS AT STAIR LANDING.
- (8) WOOD DECKING PER ARCHITECTURAL.
- TYPICAL: FRAME COLUMN CONTINUOUS THROUGH FLOOR. PROVIDE SUPPORT AROUND OPENING PER TYPICAL DETAIL. FRAME TIGHT AROUND COLUMN, ALL SIDES.

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JOHNSON RESIDENCE 15-3A RESOLUTION POINTE ANCHORAGE, ALASKA FIRST FLOOR FRAMING PLAN

DANIEL M. FOLMAR

No. CE11421

No. CE11421

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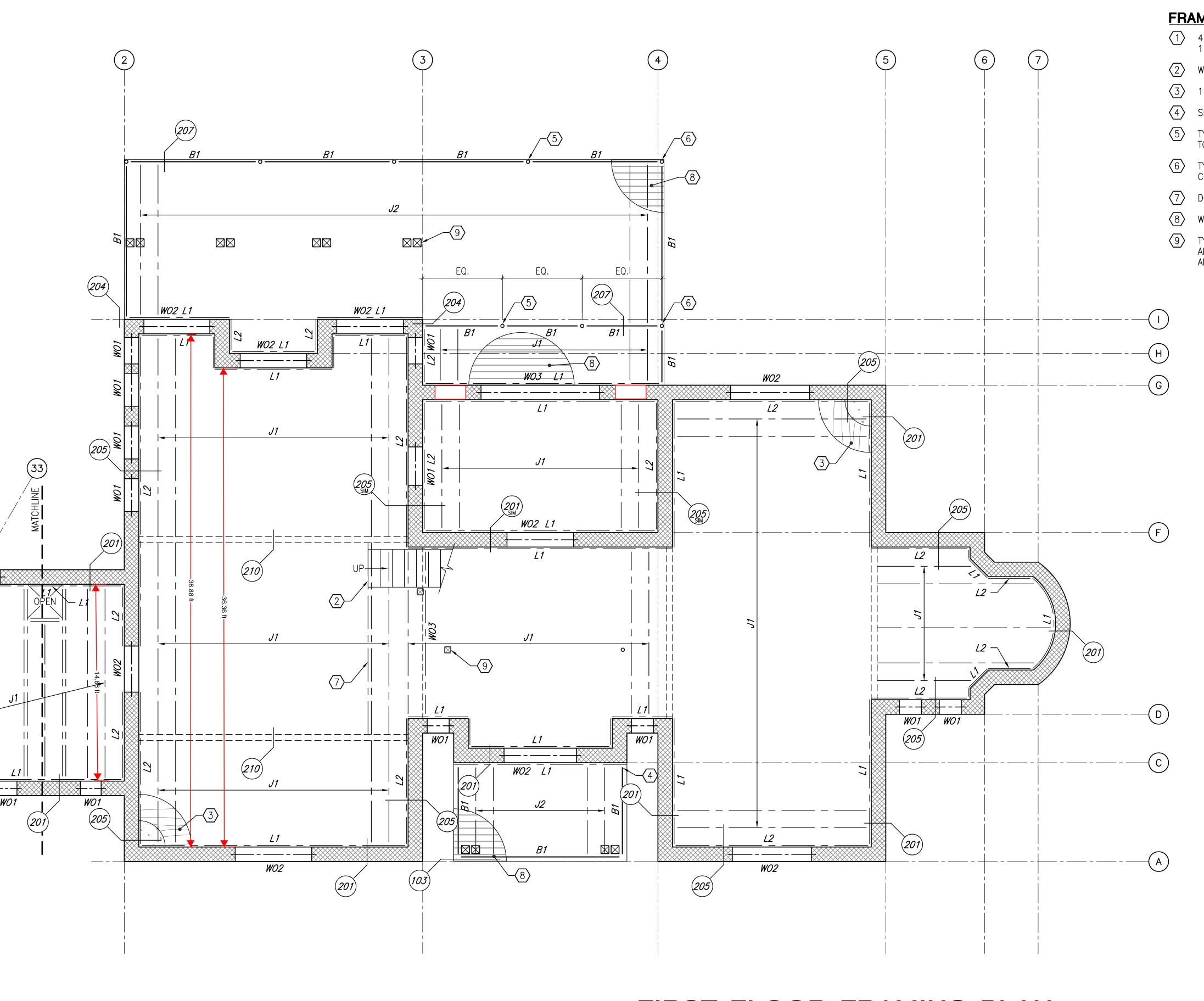
 engineer
 W. OVIATT

 drafter
 R. BRANCH

 date
 06/17/15

Sheet 06/17/15

FIRST FLOOR FRAMING PLAN
SCALE: 1/4"=1'-0"



FRAMING KEYNOTES:

- 4" CONCRETE SLAB ON GRADE. REINFORCE WITH 6x6 W1.4-W1.4 W.W.F. 1 1/2" CLEAR FROM TOP OF SLAB.
- 2 WOOD STAIRS PER G.S.N.
- 3 1 1/2" GYPCRETE OVER FLOOR SHEATHING PER G.S.N.
- SIMPSON HGU BEAM HANGER.
- TYPICAL: SIMPSON CC TYPE WELDED COLUMN CAP AT BEAMS CONNECTION TO MINI PILES.
- TYPICAL: SIMPSON ECC TYPE WELDED COLUMN CAP AT END BEAMS CONNECTING TO MINI PILES.
- 7) DOUBLE FLOOR JOISTS AT STAIR LANDING.
- > WOOD DECKING PER ARCHITECTURAL.
- TYPICAL: FRAME COLUMN CONTINUOUS THROUGH FLOOR. PROVIDE SUPPORT AROUND OPENING PER TYPICAL DETAIL. FRAME TIGHT AROUND COLUMN,

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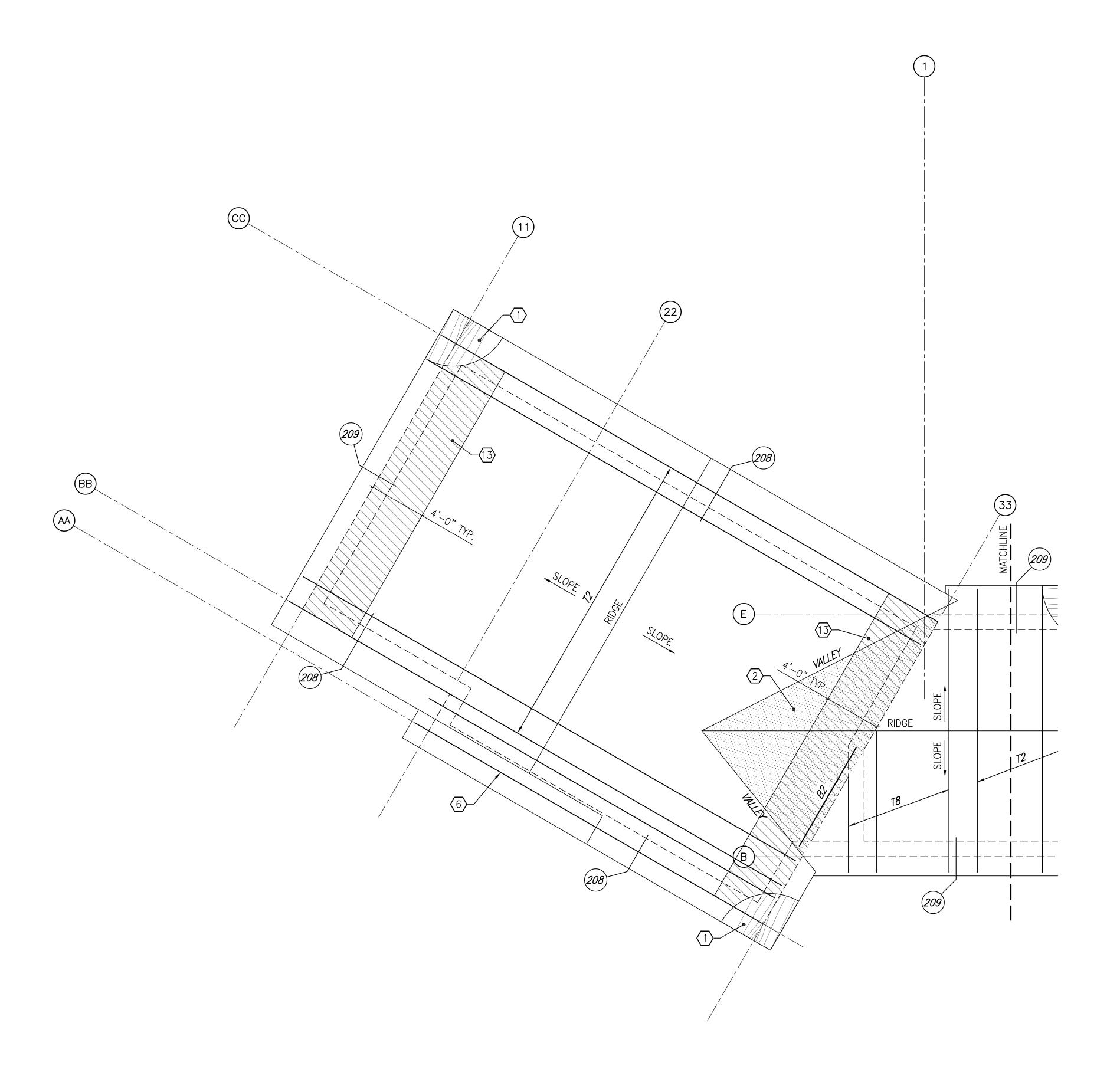
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 R. BRANCH

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 06/17/15

S2.3

FIRST FLOOR FRAMING PLAN

SCALE: 1/4"=1'-0"



SECOND FLOOR AND LOW ROOF FRAMING PLAN

SCALE: 1/4"=1'-0"

FRAMING KEYNOTES:

- 1 ROOF SHEATHING PER G.S.N.
- 2) SHADED AREA INDICATES OVERFRAMING PER TYPICAL DETAIL.
- $\overline{3}$ 3" WOOD ROOF DECKING PER G.S.N.
- 4 WOOD STAIRS PER G.S.N.
- 5 FLOOR SHEATHING PER G.S.N.
- 6 ARCHITECTURALLY EXPOSED TIMBER END TRUSS. SEE TYPICAL DETAIL FOR CONNECTION INFORMATION.
- (7) SIMPSON HGU BEAM HANGER.
- 8 POCKET BEAM INTO I.C.F. WALL. PROVIDE ROOFING FELT PROTECTION FOR BEAM PER G.S.N.
- 9 EXTEND WOOD LEDGER TO BEAR ON I.C.F. WALL. NO SPLICE IN LEDGER WITHIN 10'-0" OF WALL CORNER.
- SIMPSON CS14 STRAP. LAP 16" ONTO BEAM, AND LAP OTHER END 4'-0" ONTO FLAT 2x BLOCKING BETWEEN JOIST BAYS. FASTEN WITH 10d NAILS.
- 11) SIMPSON CS14 STRAP. LAP EACH END 16" ONTO BOTTOM OF BEAM/TRUSS FASTEN WITH 10d NAILS.
- (12) ALIGN A TRUSS WITH BEAM BEYOND.
- CROSS HATCHING INDICATES FLAT 2x BLOCKING. BLOCKING IS REQUIRED AT ALL DIAPHRAGM PANEL EDGES AS INDICATED. PROVIDE EDGE ATTACHMENT PER G.S.N.
- (14) EXTEND CANTILEVERED BEAM TIGHT TO TG8, TO INSTALL STRAP WITH TRUSS BEYOND.

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JOHNSON RESIDENCE 15-3A RESOLUTION POINTE ANCHORAGE, ALASKA FIRST FLOOR FRAMING PLAN



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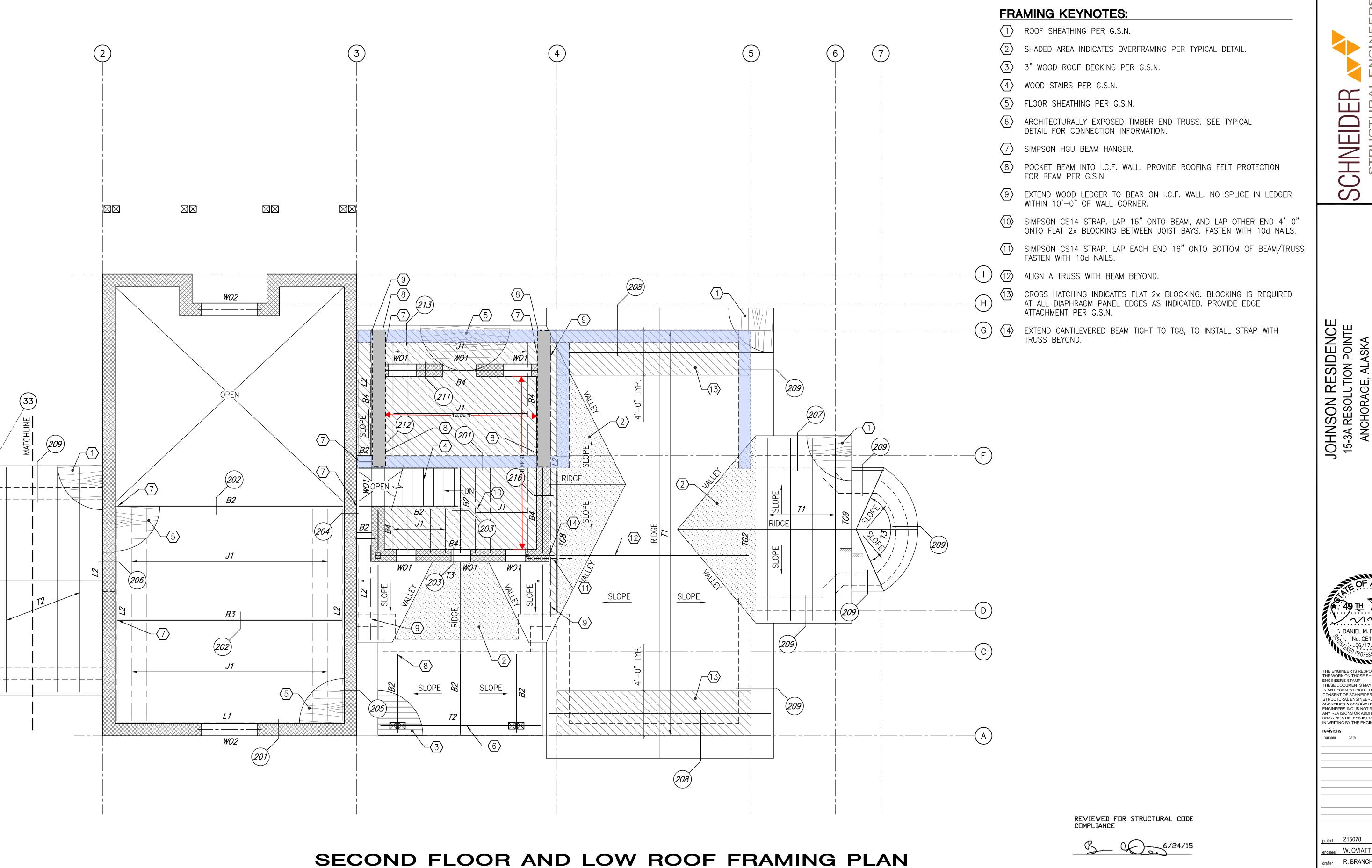
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engineer W. OVIATT

<u>drafter</u> R. BRANCH date 06/17/15

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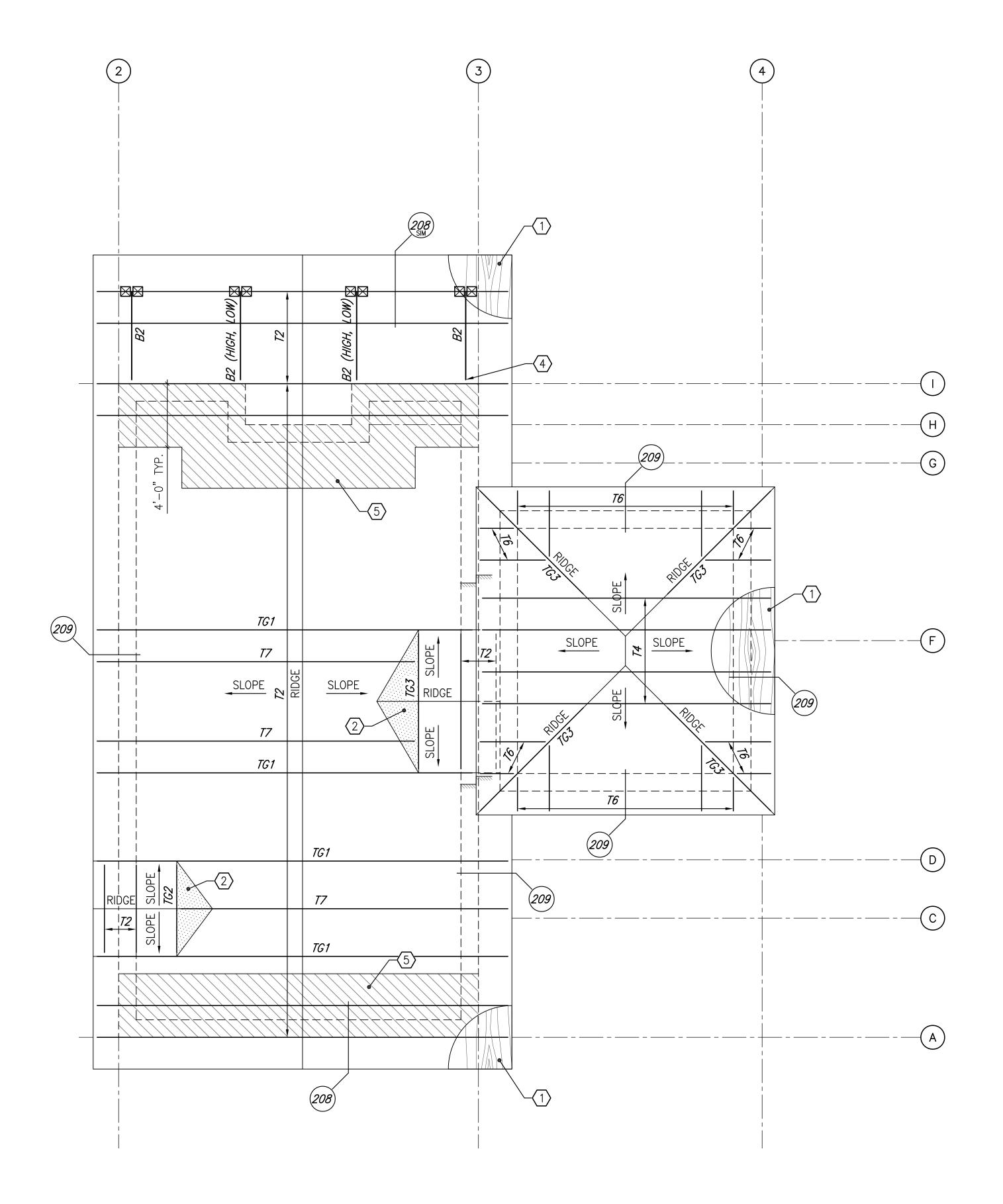


SCALE: 1/4"=1'-0"

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project 215078 engineer W. OVIATT

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HIGH ROOF FRAMING PLAN

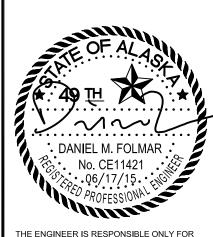
SCALE: 1/4"=1'-0"

FRAMING KEYNOTES:

- 1 ROOF SHEATHING PER G.S.N.
- SHADED AREA INDICATES OVERFRAMING PER TYPICAL DETAIL.
- ARCHITECTURALLY EXPOSED TIMBER END TRUSS. SEE TYPICAL DETAIL FOR CONNECTION INFORMATION.
- TYPICAL: POCKET BEAM INTO I.C.F. WALL. PROVIDE ROOFING FELT PROTECTION FOR BEAM PER G.S.N.
- CROSS HATCHING INDICATES FLAT 2x BLOCKING. BLOCKING IS REQUIRED AT ALL DIAPHRAGM PANEL EDGES AS INDICATED. PROVIDE EDGE ATTACHMENT PER G.S.N.

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JOHNSON RESIDENCE 15-3A RESOLUTION POINTE ANCHORAGE, ALASKA

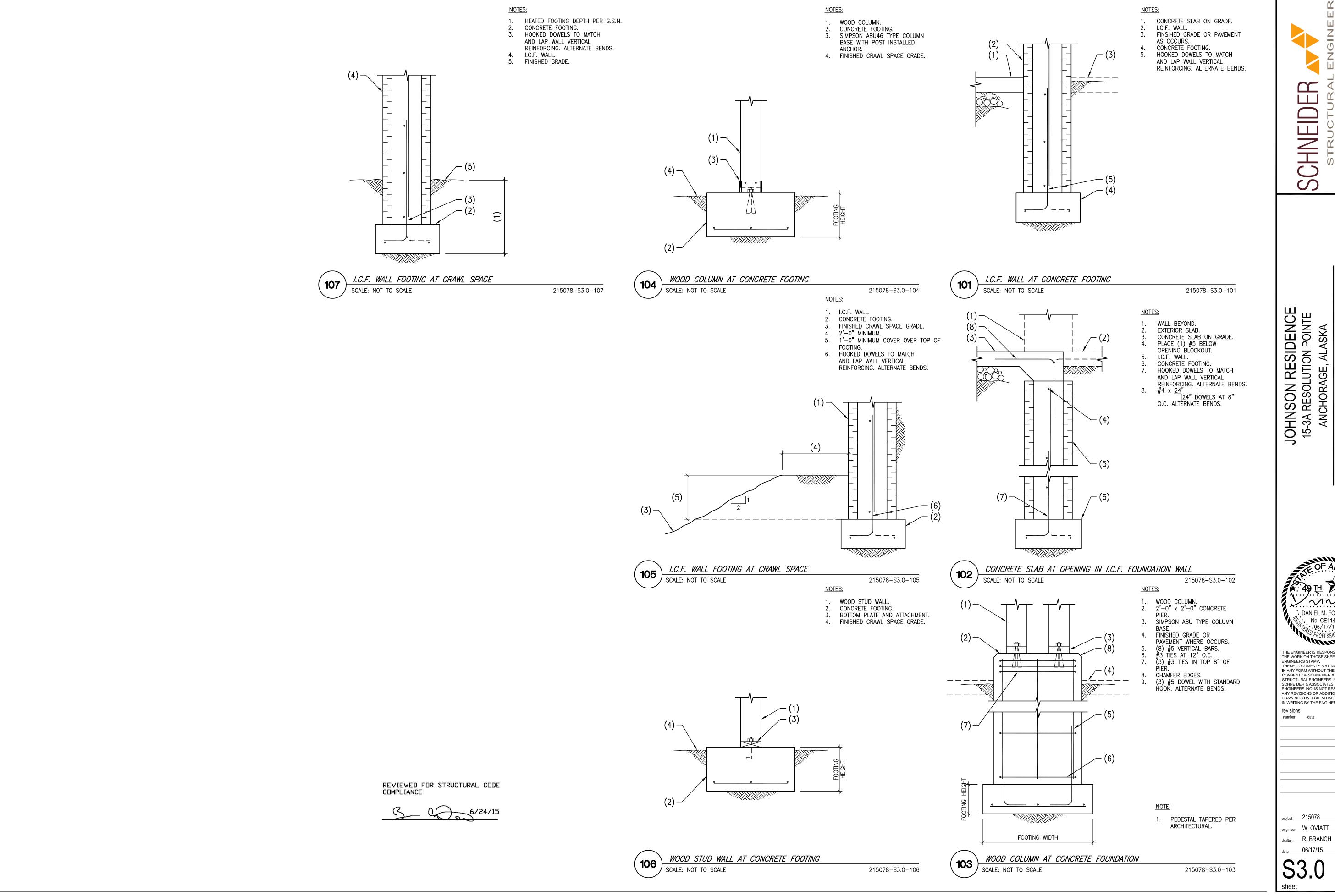


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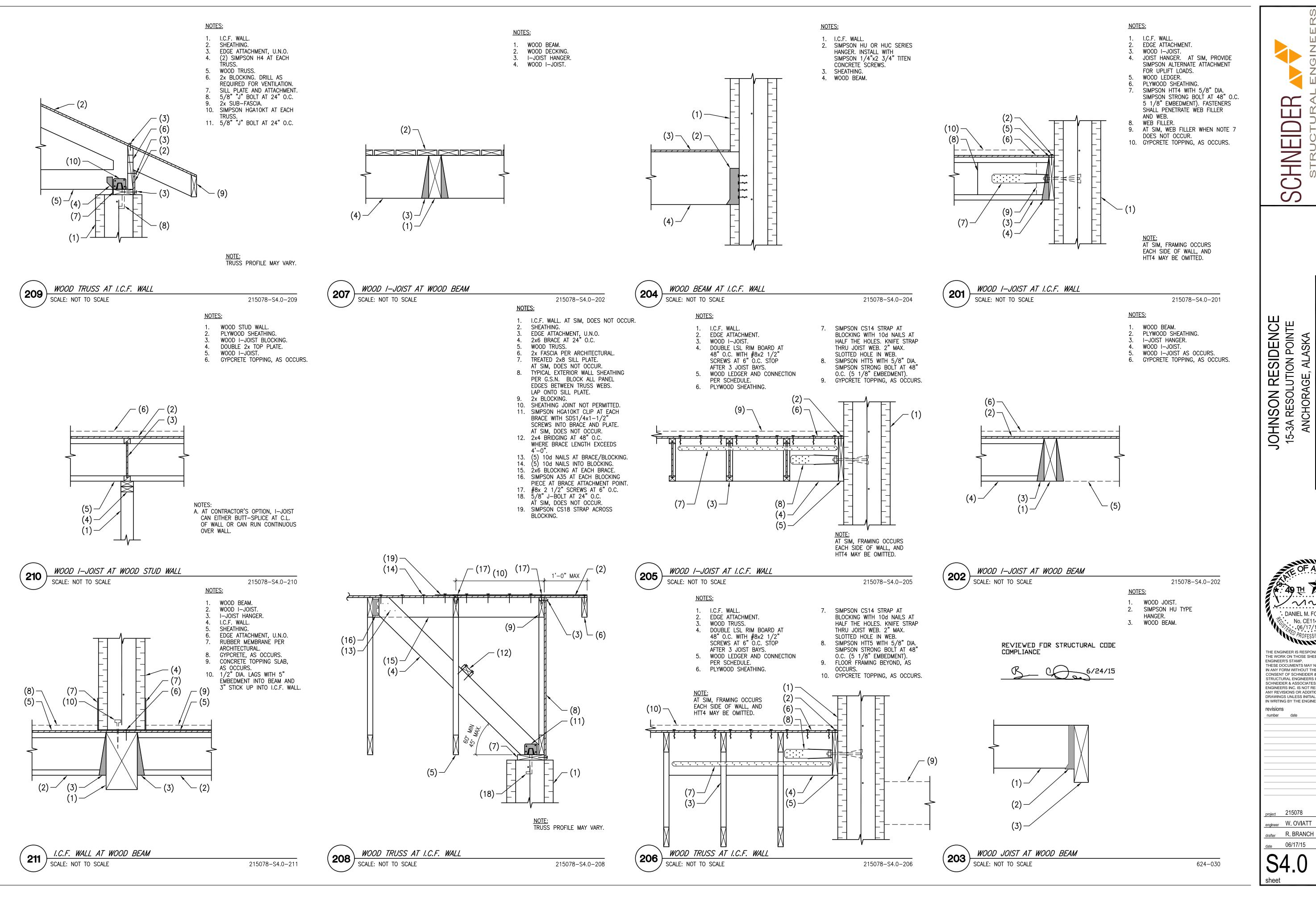


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06/17/15



No. CE11421 PROFESSIONA

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drafter R. BRANCH 06/17/15

S4.0

NOTES:
A. AT CONTRACTOR'S OPTION, I—JOIST
CAN EITHER BUTT—SPLICE AT C.L.
OF WALL OR CAN RUN CONTINUOUS
OVER WALL.

NOTES:

WOOD STUD WALL.

PLYWOOD SHEATHING.
WOOD I-JOIST BLOCKING.
DOUBLE 2x TOP PLATE.
WOOD I-JOIST.

WOOD I-JOIST AT WOOD STUD WALL SCALE: NOT TO SCALE

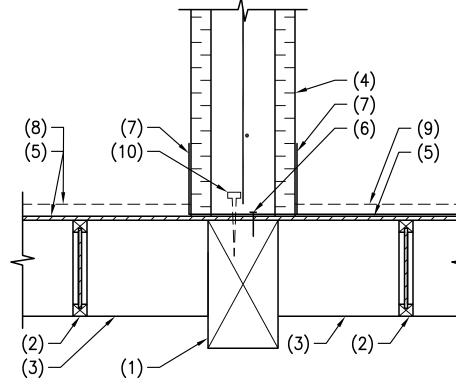
REVIEWED FOR STRUCTURAL CODE COMPLIANCE

215078-S4.0-215

NOTES:

- WOOD BEAM. WOOD I-JOIST.
- 2. WOOD 1-JOIST.
 3. WOOD LSL RIM BOARD AT
 48" O.C. WITH #8x2 1/2"
 SCREWS AT 6" O.C. STOP
 AFTER 3 JOIST BAYS.
 4. I.C.F. WALL.
 5. SHEATHING.
 6. EDGE ATTACHMENT, U.N.O.
- RUBBER MEMBRANE PER ARCHITECTURAL.
 GYPCRETE, AS OCCURS.
 CONCRETE TOPPING SLAB,
- AS OCCURS.

 10. 1/2" DIA. LAGS WITH 5"
 EMBEDMENT INTO BEAM AND
 3" STICK UP INTO I.C.F. WALL.



I.C.F. WALL AT WOOD BEAM SCALE: NOT TO SCALE

215078-S4.0-212

NOTES: I.C.F. WALL. EDGE ATTACHMENT.

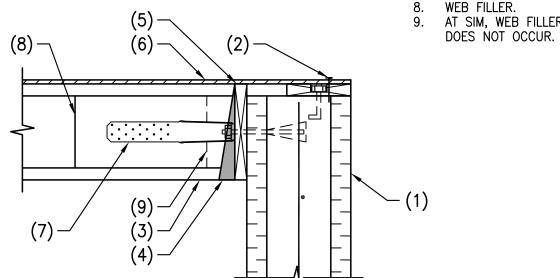
WOOD I-JOIST. JOIST HANGER. AT SIM, PROVIDE

JOIST HANGER. AT SIM, PROVIDE SIMPSON ALTERNATE ATTACHMENT FOR UPLIFT LOADS.
 WOOD LEDGER.
 PLYWOOD SHEATHING.
 SIMPSON HTT4 WITH 5/8" DIA.
 SIMPSON STRONG BOLT AT 48" O.C.
 1/8" EMBEDMENT). FASTENERS SHALL PENETRATE WEB FILLER

AND WEB.

8. WEB FILLER.

9. AT SIM, WEB FILLER WHEN NOTE 7



WOOD I-JOIST AT I.C.F. WALL SCALE: NOT TO SCALE

NOTES:

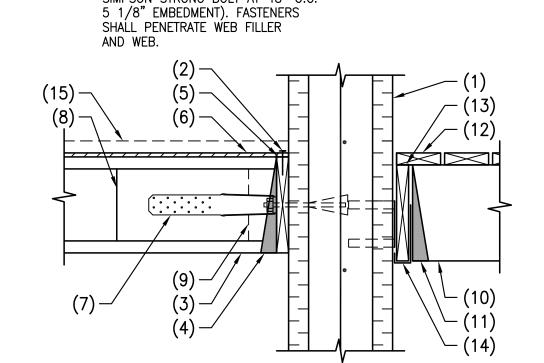
I.C.F. WALL. EDGE ATTACHMENT. WOOD I-JOIST. 4. JOIST HANGER. AT SIM, PROVIDE SIMPSON ALTERNATE ATTACHMENT

FOR UPLIFT LOADS. WOOD LEDGER. PLYWOOD SHEATHING. SIMPSON HTT4 WITH 5/8" DIA. SIMPSON STRONG BOLT AT 48" O.C.

215078-S4.0-201 WEB FILLER.

AT SIM, WEB FILLER WHEN NOTE 7 DOES NOT OCCUR. 10. 2x DECK JOIST. 11. JOIST HANGER.

12. 2x DECKING. 13. WOOD LEDGER. 14. LEDGER CONNECTION. 15. GYPCRETE TOPPING, AS OCCURS.



WOOD I-JOIST AT I.C.F. WALL SCALE: NOT TO SCALE

215078-S4.0-214

JOHNSON RESIDENCE 15-3A RESOLUTION POINTE ANCHORAGE, ALASKA

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number date

project 215078 engineer W. OVIATT drafter R. BRANCH

06/17/15 sheet